

Warsaw, 22 - 23 October 2025

II MIĘDZYNARODOWA KONFERENCJA

Nowoczesne nawierzchnie drogowe - recykling i dekarbonizacja

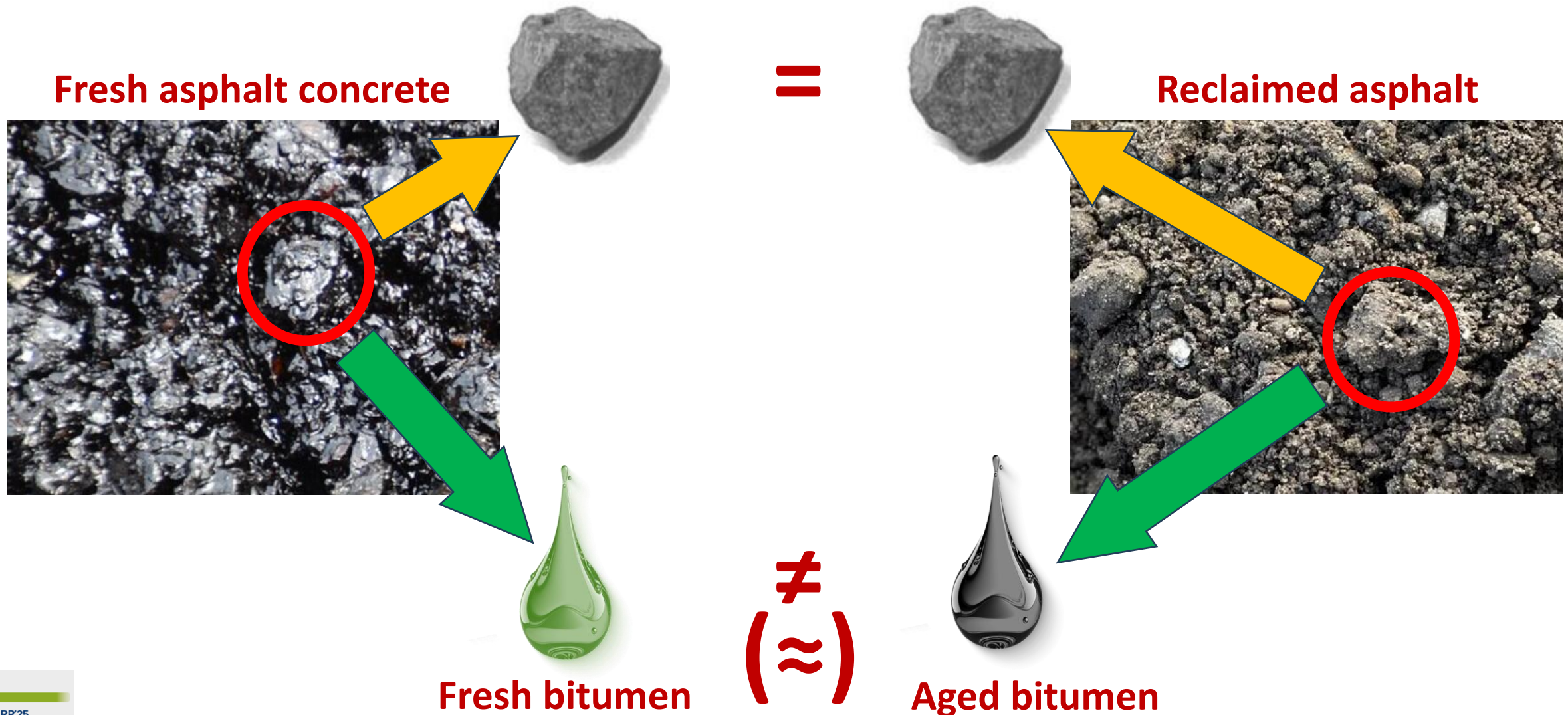
II INTERNATIONAL CONFERENCE

Modern road pavements - recycling and decarbonization

Recycling of Reclaimed Asphalt
Basic Concepts and new perspectives

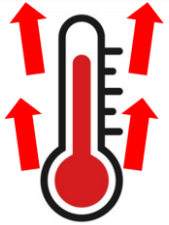
Gabriele Tebaldi, PhD, PE, AAPT Fellow, Rilem Fellow

Asphalt Recycling



Asphalt Recycling

Hot recycling



Aggregates



+

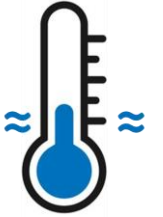


Still available + activable
bitumen

Key Concept

Use “old bitumen” and “old aggregates” instead of new ones

Cold recycling



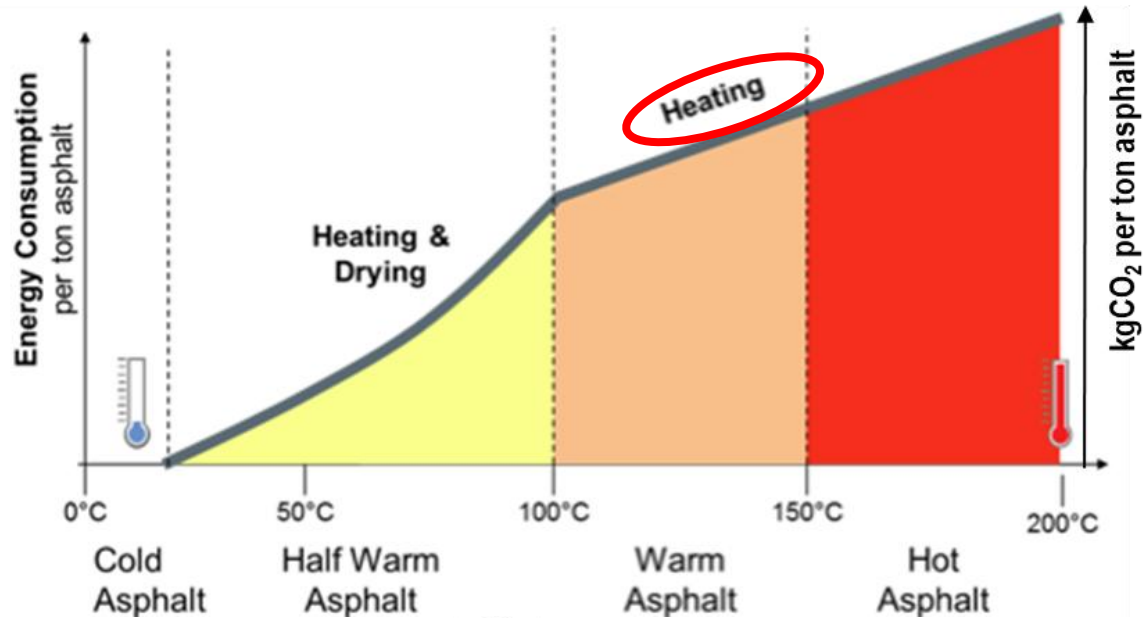
“≈ Black rock”



Key Concept

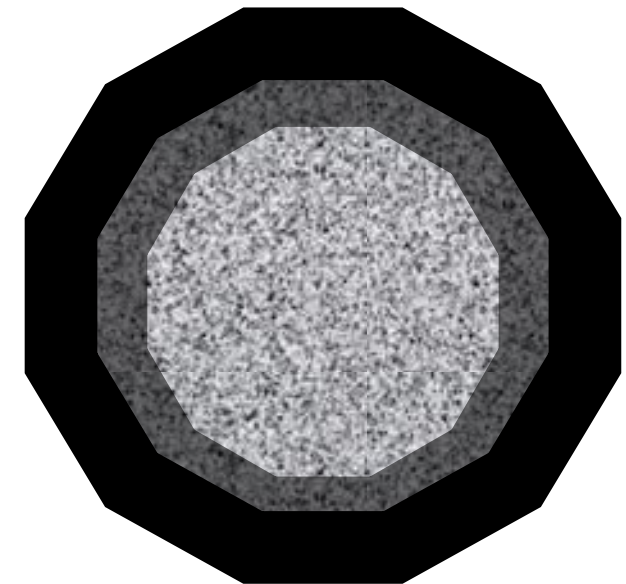
Use RAP almost like aggregates without heating

Asphalt Recycling



Hot recycling

Heating the bitumen reduces its viscosity, allowing it to coat the hot aggregates and provide the lubrication necessary for compaction.



Cold recycling ≈  ≈

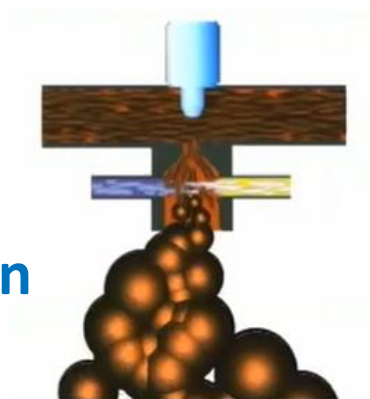
In cold recycling techniques, there is no aggregate heating and it's not possible to count on the fluid behavior of the bitumen; therefore, a carrier for the bitumen and a lubricant are needed.

Cold Asphalt Recycling



Foam asphalt

The carrier of the bitumen are dust and active filler

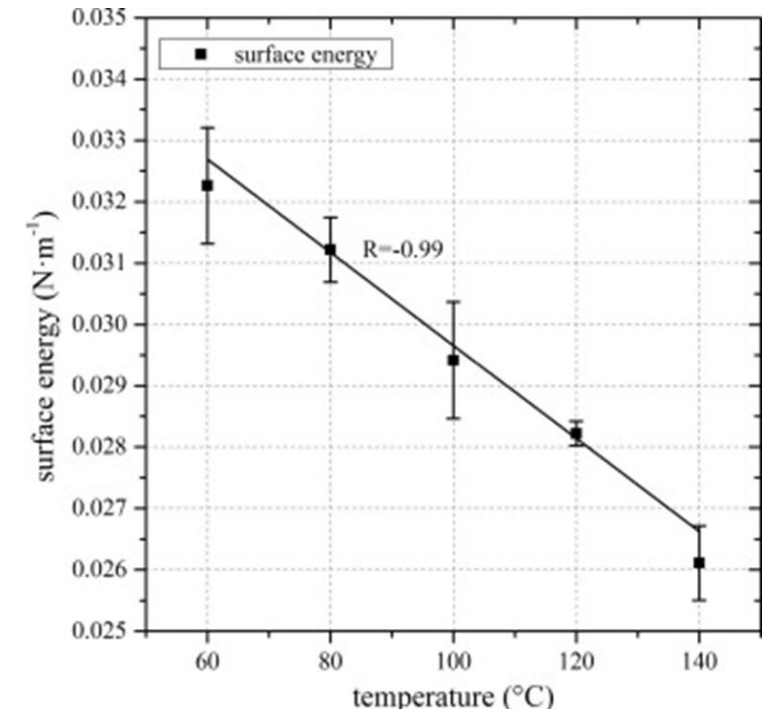
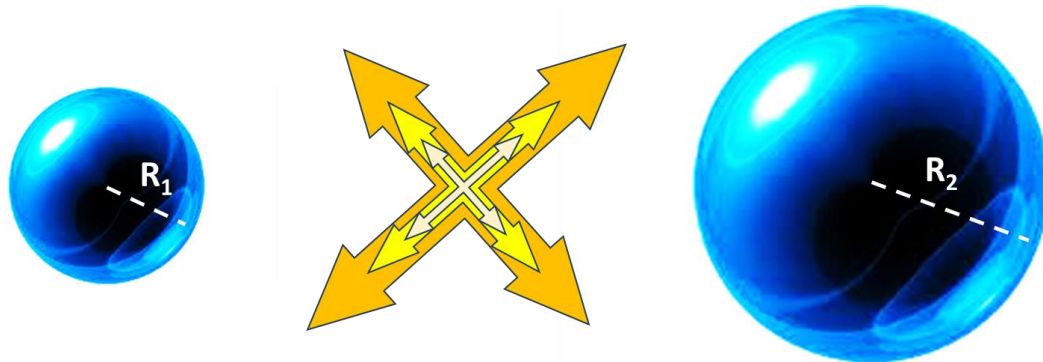


Bituminous emulsion
the carrier of the bitumen is the water

Foam bitumen

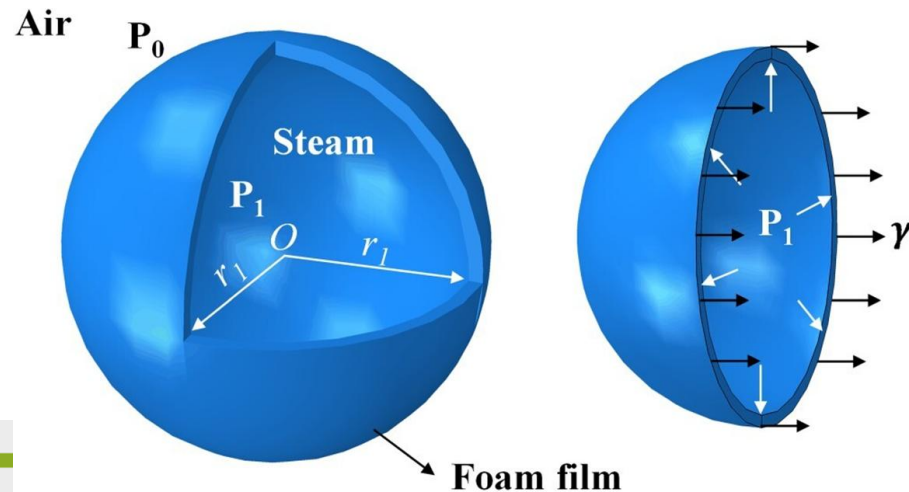
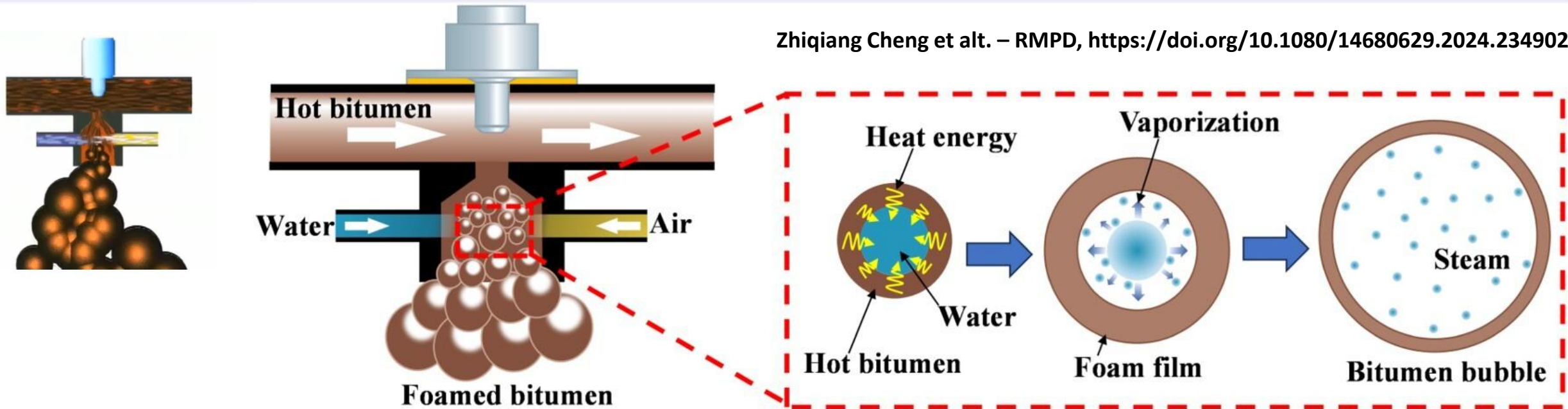
Surface energy, a fundamental intrinsic property of materials, is defined as the amount of work done to increase a unit surface area of a substance, or the potential energy per unit area of molecules on the substance's surface over molecules inside

Bitumen surface energy decreases with increasing temperature. This is because at higher temperatures, the intermolecular forces within the bitumen weaken, leading to a reduction in the energy required to create or disrupt the surface.



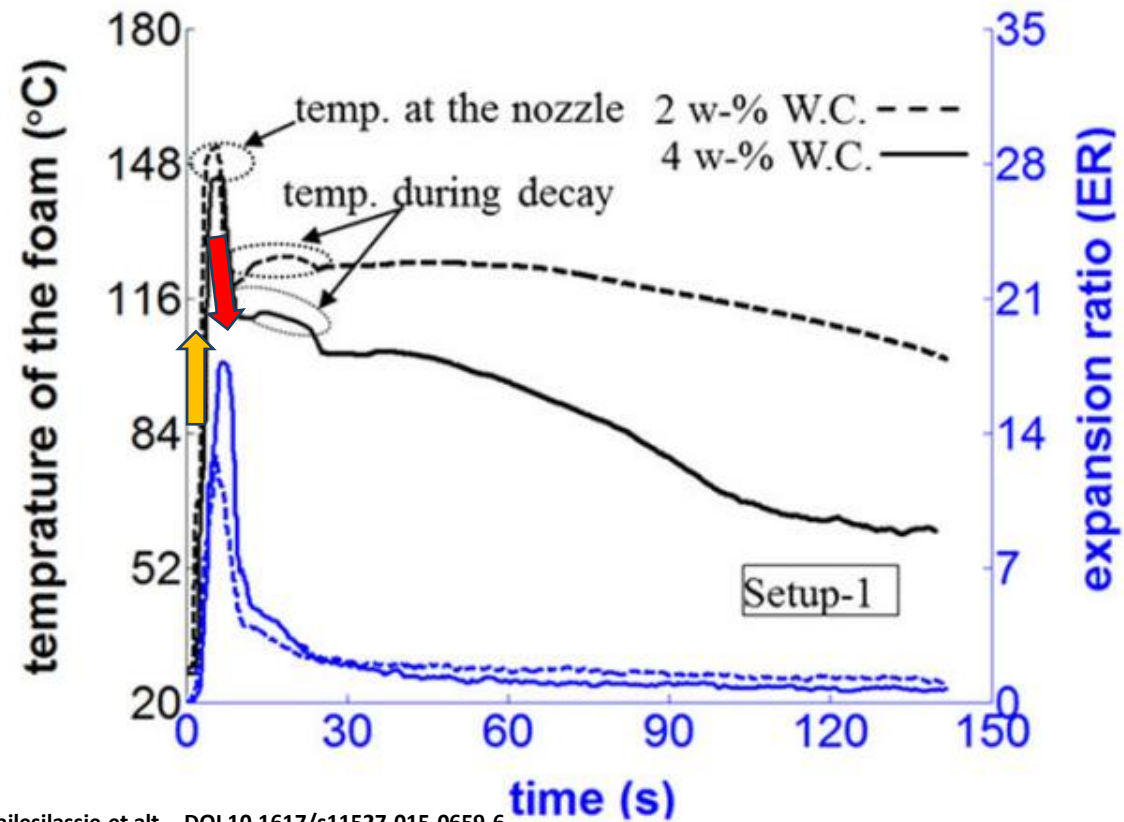
Foam Bitumen

Zhiqiang Cheng et al. – RMPD, <https://doi.org/10.1080/14680629.2024.2349026>

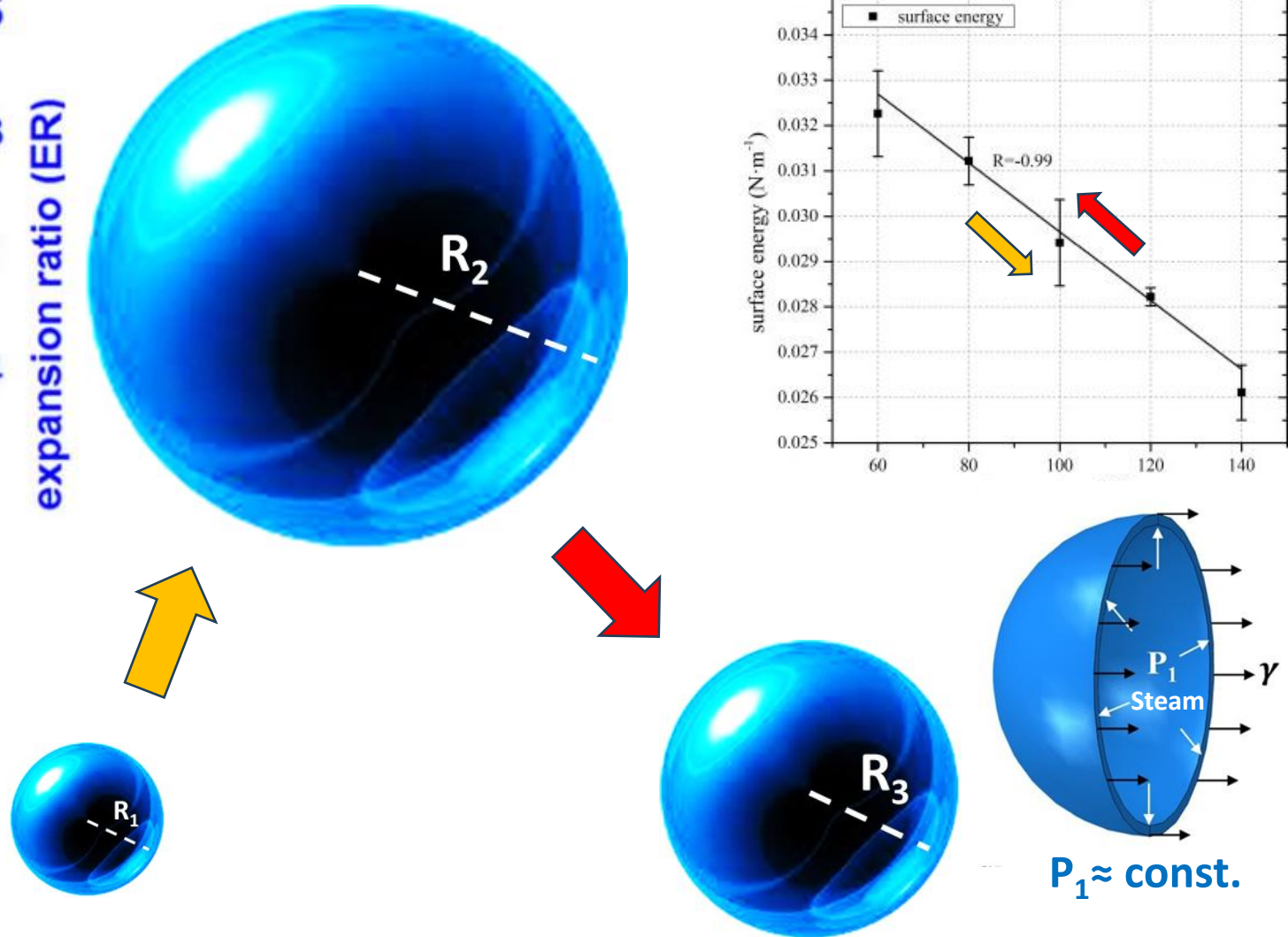


The evolving steam volume and pressure adhere to the ideal-gas law. Thus, as the bubble expands, its internal pressure reduces until equilibrium between internal and external pressure is achieved.

Foam Bitumen

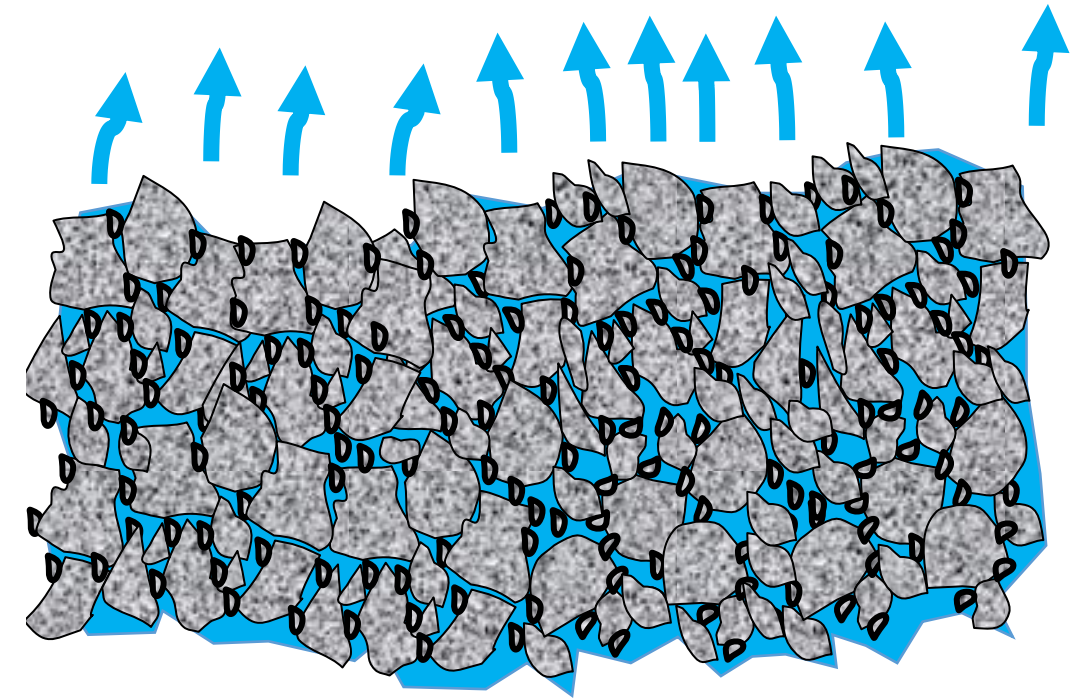
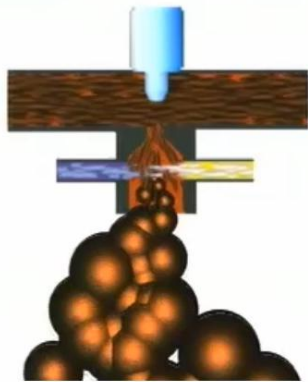


Hailesilassie et al. - DOI 10.1617/s11527-015-0659-6



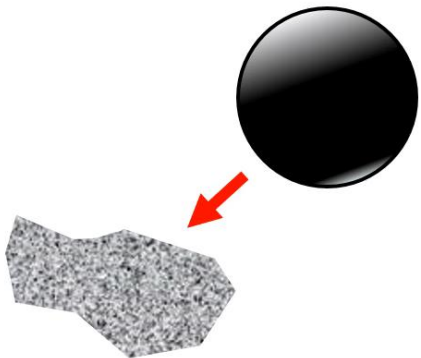
Foam Bitumen

The fine particles that catch the bitumen droplets are the elements that partially bind the aggregates among them

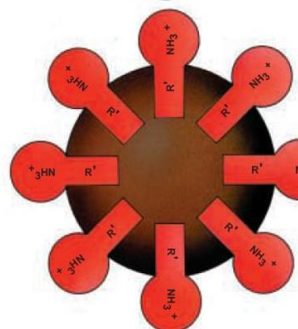
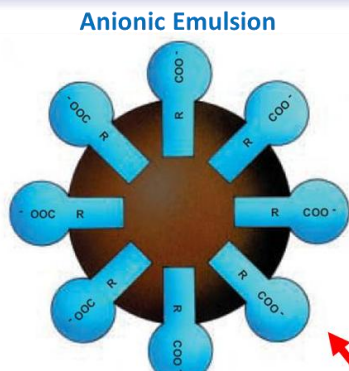
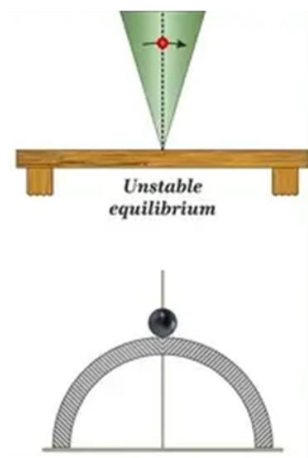


The total water content (foaming water + additional water) provides the lubricant effect before leaving during the curing phase

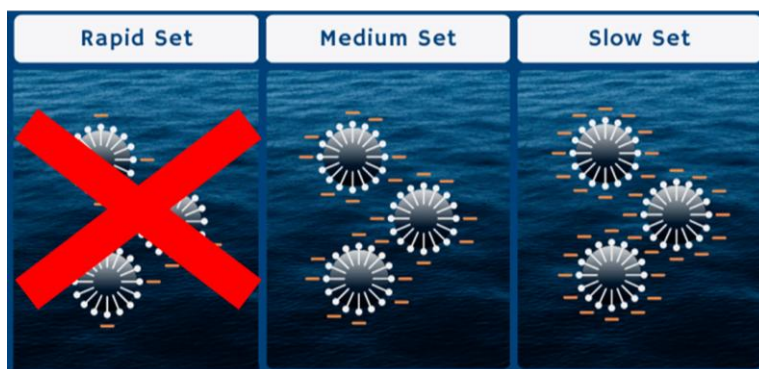
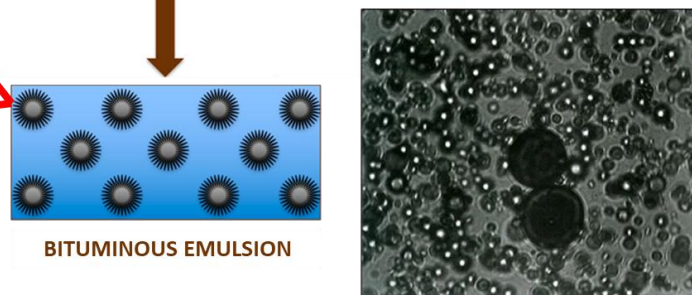
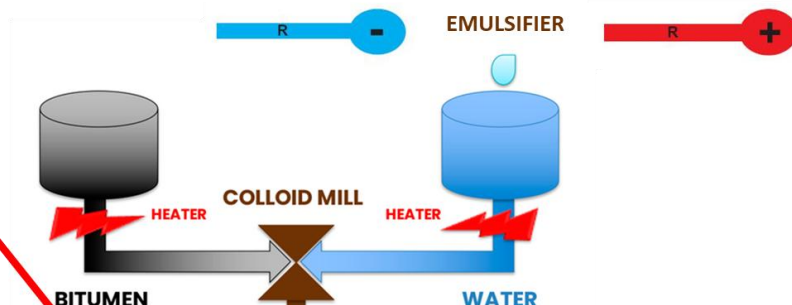
The blasting and the dust particles are the carriers of bitumen



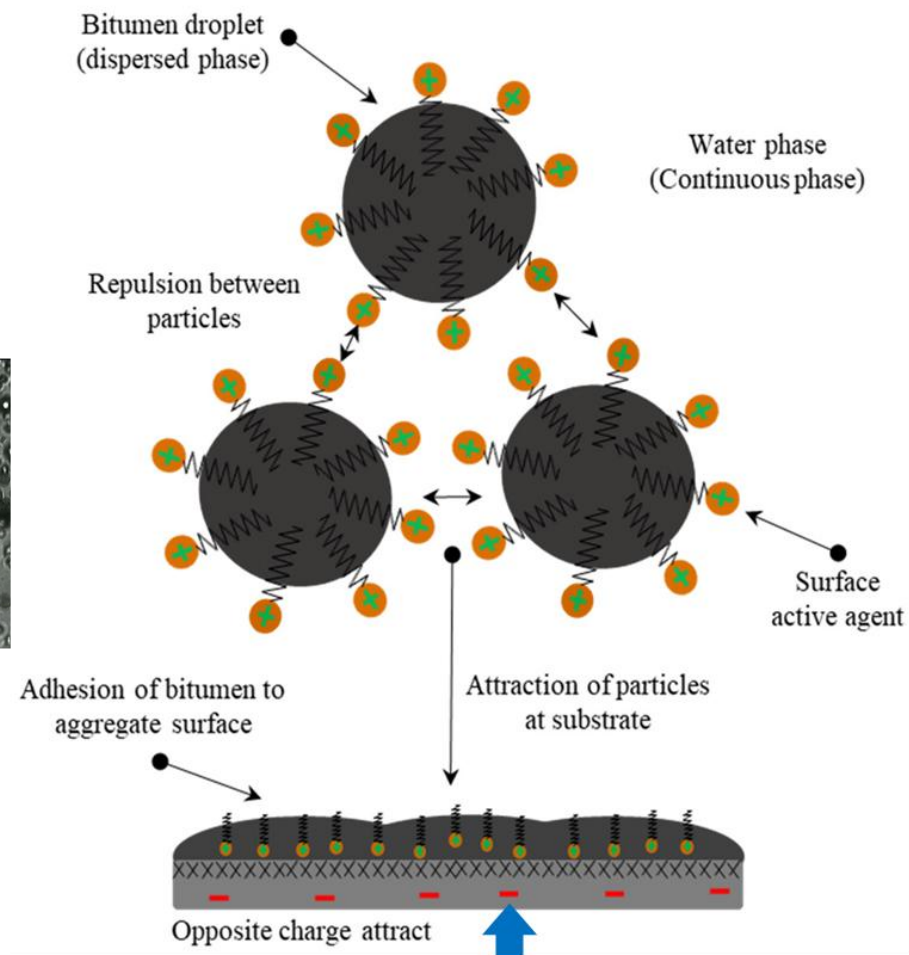
Bituminous Emulsion



Emulsion: a fine dispersion of minute droplets of one liquid in another in which it is not soluble or miscible



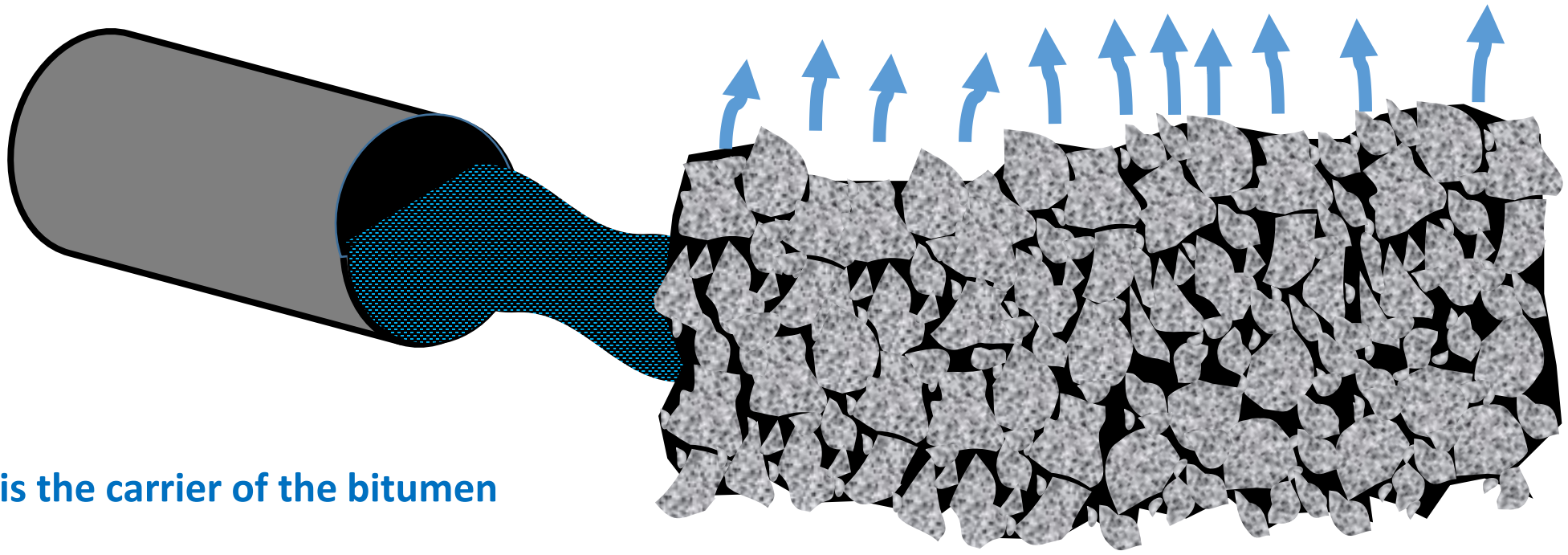
Emulsion breaking process



The choice of the emulsifier is function of the aggregates' characteristics

Bituminous Emulsion

The total amount of fluid (emulsion + additional water) provides the lubricant effect before leaving during the curing phase



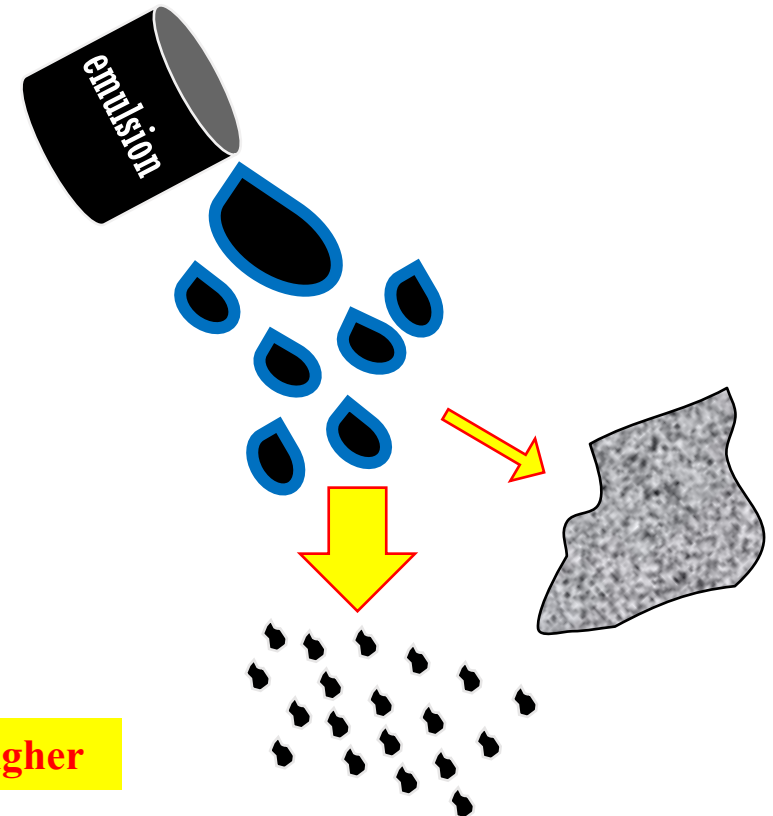
The water is the carrier of the bitumen

Bituminous Emulsion

Because the surface area of the fines aggregates it is much bigger than the surface area of coarse aggregates, the emulsion will be concentrated on the fines fraction.

Particle size (mm)	Surface area factor (m ² /kg)
19	0.13
13.2	0.18
9.5	0.24
6.7	0.31
4.75	0.43
2.36	0.82
1.18	1.64
0.6	2.87
0.3	6.14
0.15	12.24
0.075	32.77

252 times higher

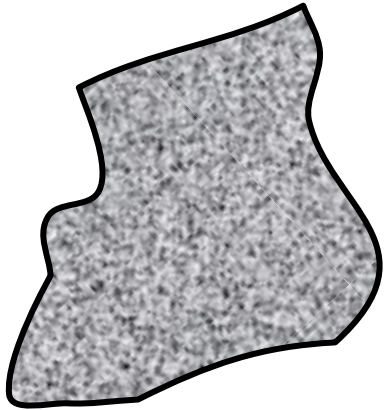


Bituminous Emulsion

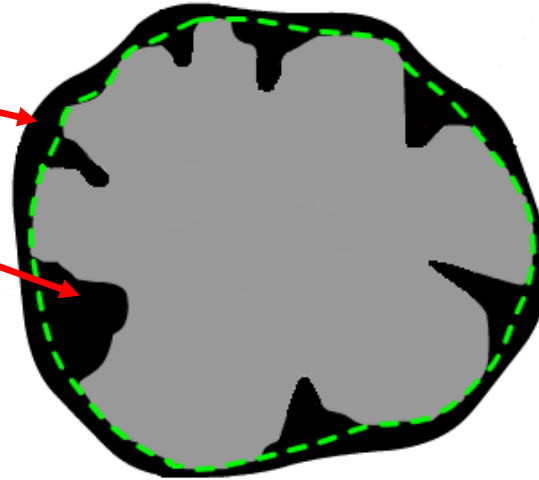
Effective asphalt film

Absorbed asphalt

Non-continuously bounded material

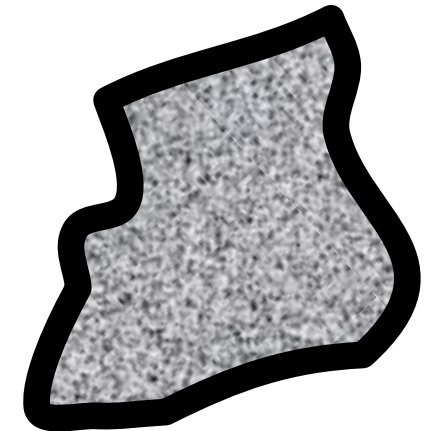


Bituminous emulsion
stabilized material



A bituminous emulsion stabilized materials has the aggregates fully covered by the emulsions, but the film made by the emulsion (in total around 2-3% of the weight of aggregates) it is not enough thick to make a fully bounded material

Continuously-bounded material



Hot asphalt mixture

Bituminous Stabilized Materials (BSM)

BSMs are partially bounded materials

The bitumen carried by fine aggregates (mainly by filler) it makes an adhesive mastic disperse inside the mixture

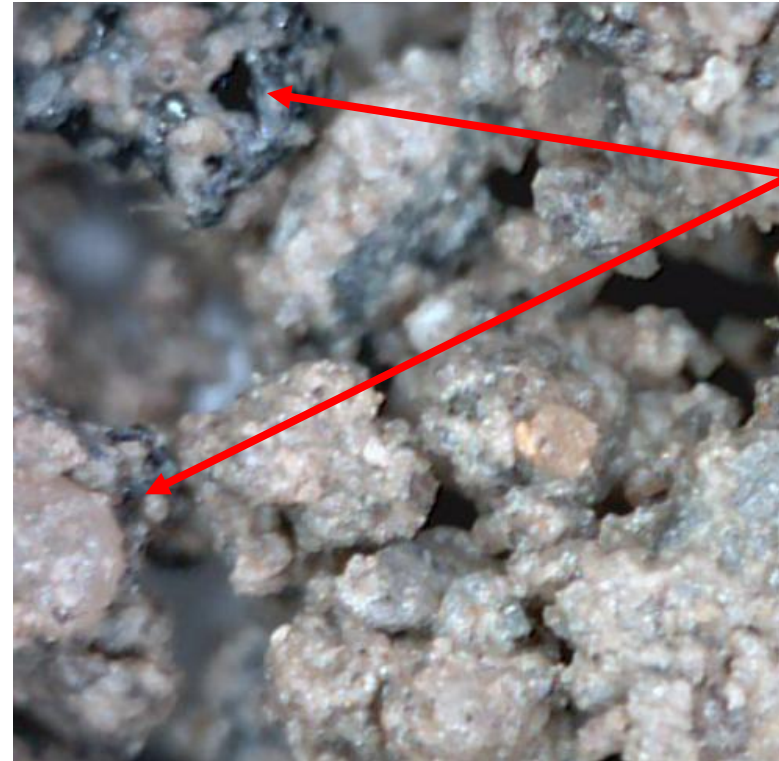
Emulsion

Partially coated aggregates
or non uniformly
coated aggregates

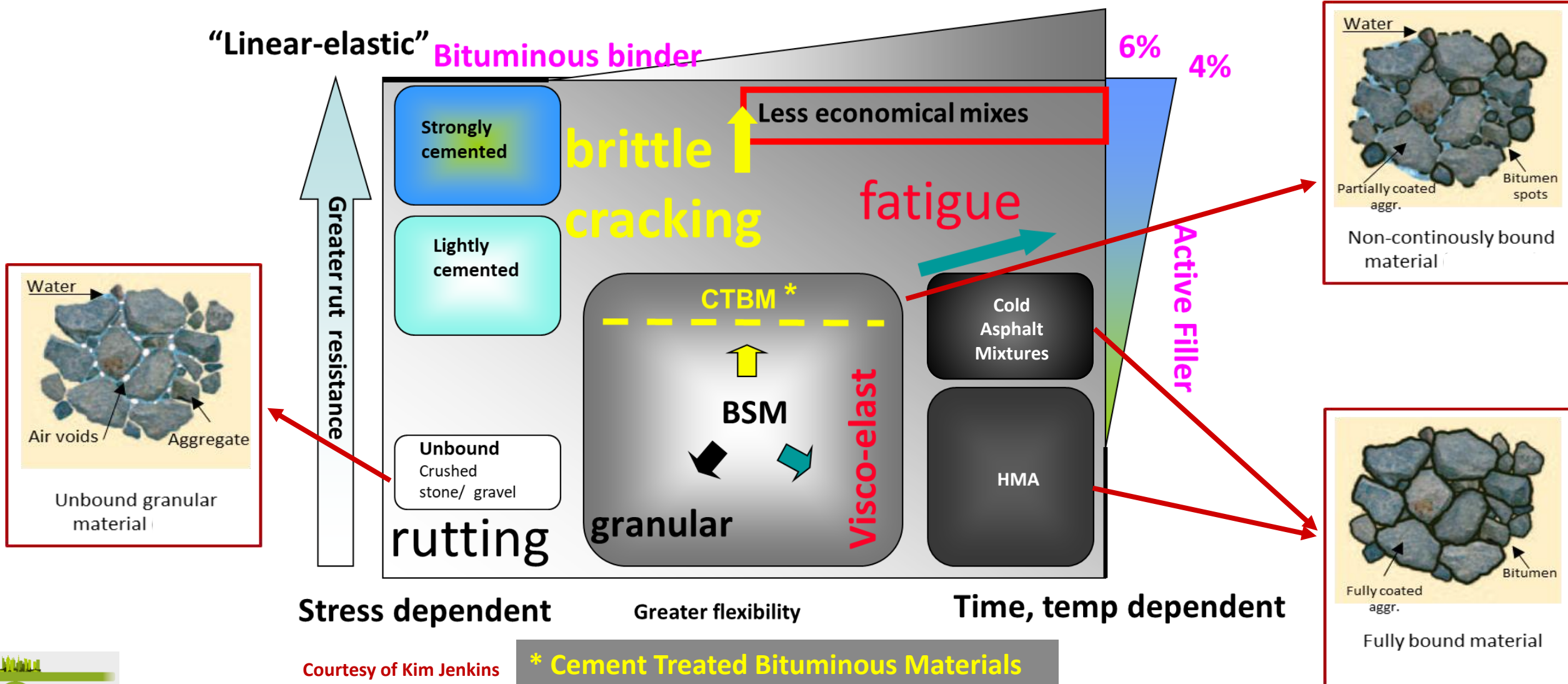


Foam

Bitumen spots
bitumen
absorbed by
filler of bitumen
drops attached
on the
aggregates'
surface

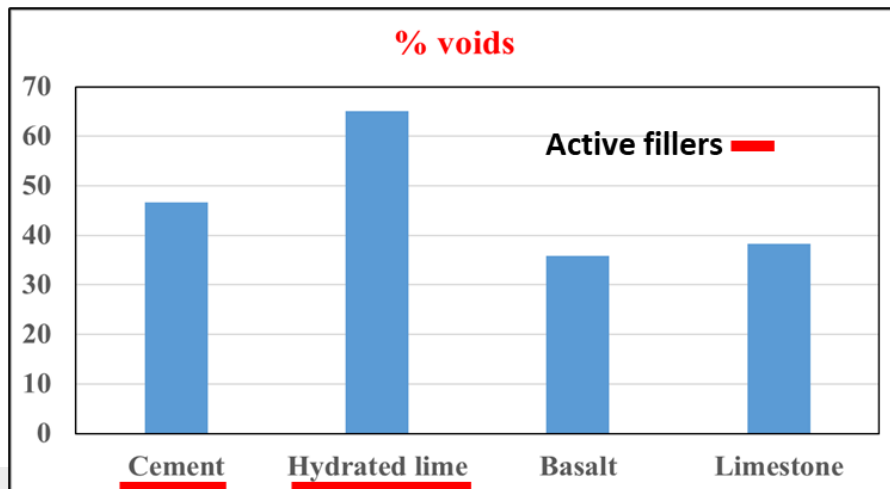
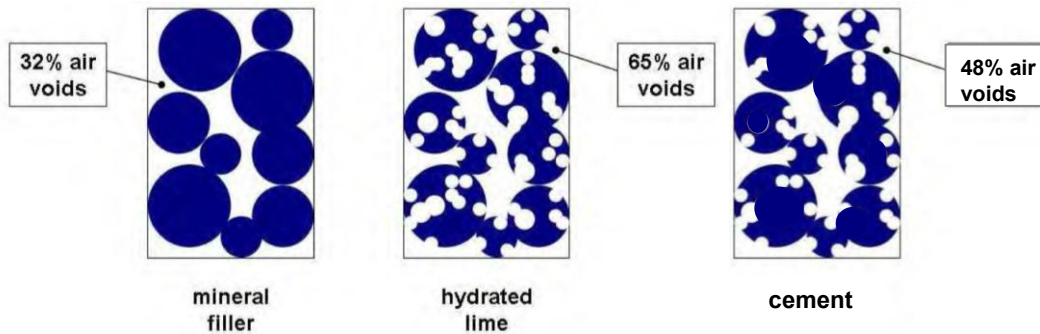


Bituminous Stabilized Materials (BSM)



BSM & active fillers

Active filler it is a filler (mineral element part of the aggregates passing at sieve 0.075/0.062) that is chemically active. The most common active fillers are lime, cement and fly ash.

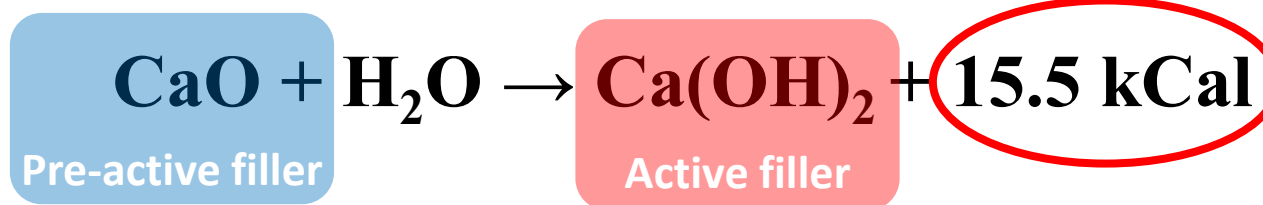
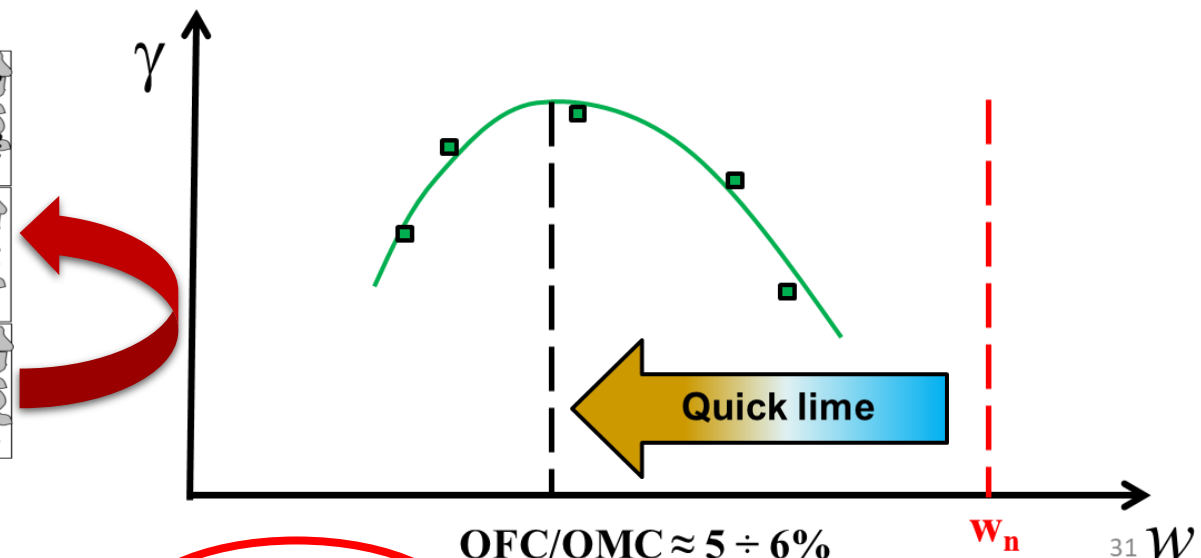
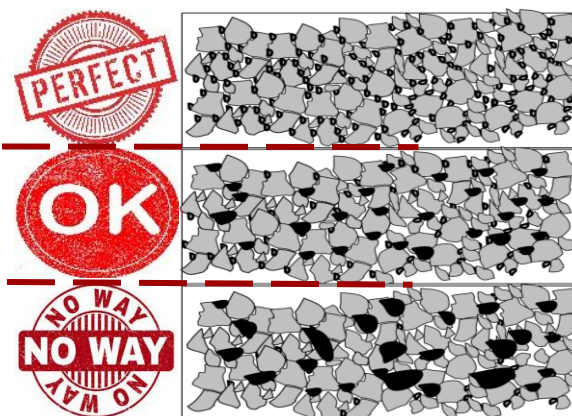
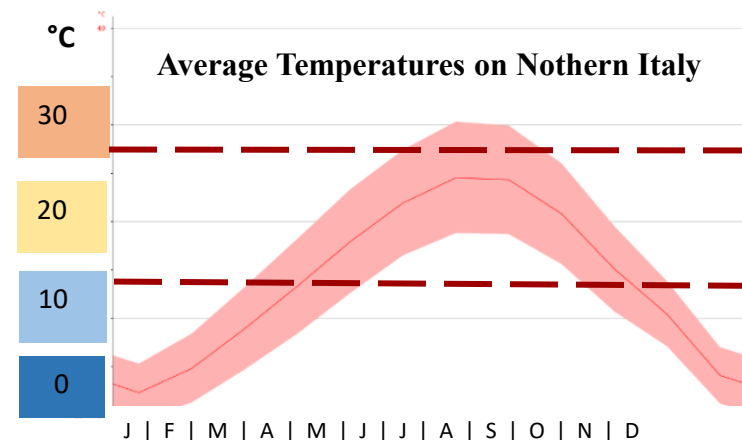


The purpose of incorporating active filler in BSM is to:

- Improve dispersion of the bitumen in the mix
- Accelerate curing of compacted mix
- Control emulsion's breaking time
- Increase stiffness & strength of mix → setting & hardening
- Improve adhesion of the bitumen to the aggregate
 - ↳ mainly lime with acid aggregates: it makes basic salts that are able to avoid the "acid-acid contact" between bitumen and aggregate surface

Active fillers: collateral physical effects

Heating



High electrochemical potential & high-speed reaction

Pre-active filler: a material that became active filler for a chemical or physical immediate change during the mixing phase

Use of calcium oxide as active filler for bituminous stabilised materials

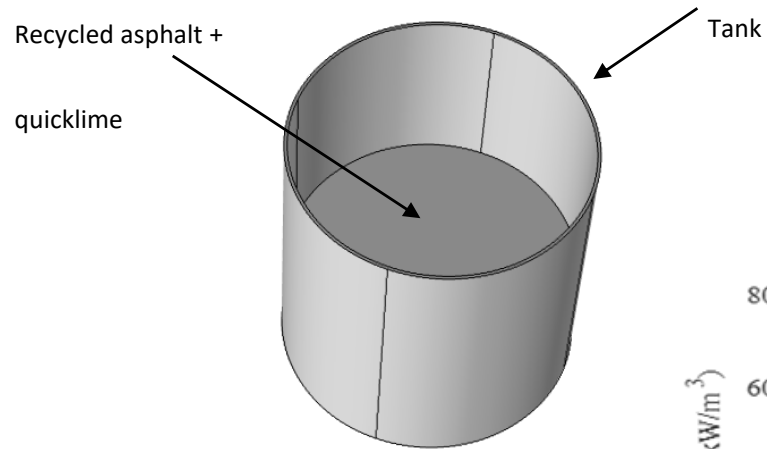
Beatriz Chagas Silva Gouveia, Francesco Preti, Romeo, Kim Jenkins, Gabriele Tebaldi

RMPD <https://doi.org/10.1080/14680629.2021.1881591>

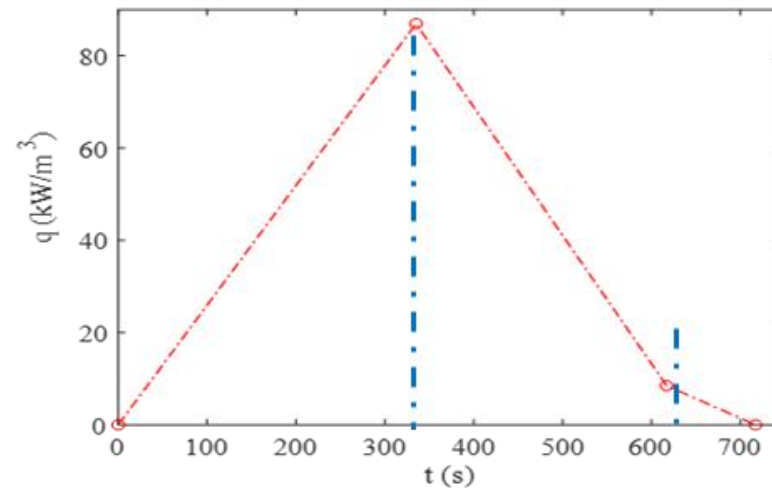
Active fillers: collateral physical effects

Heating

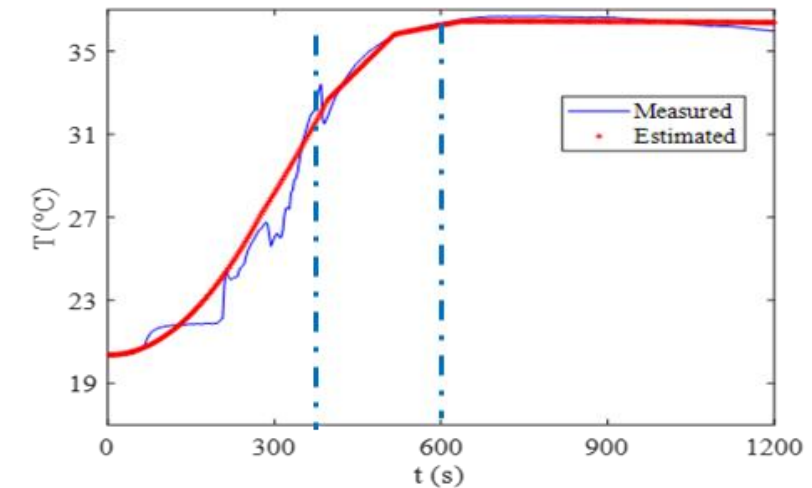
Emulsion + Quicklime
Effects of temperature



3-D numerical finite element model of the whole test body (tank and volume of material) was implemented within the Comsol Multiphysics® environment



Function of heat releasing



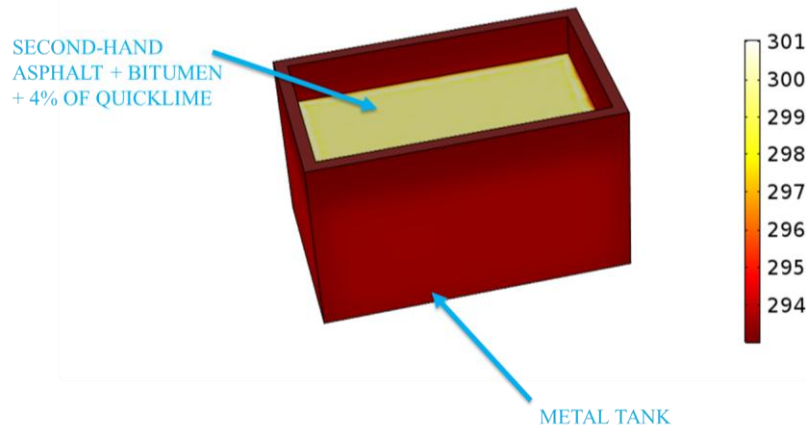
Estimated vs. measured T distribution

Active fillers: collateral physical effects

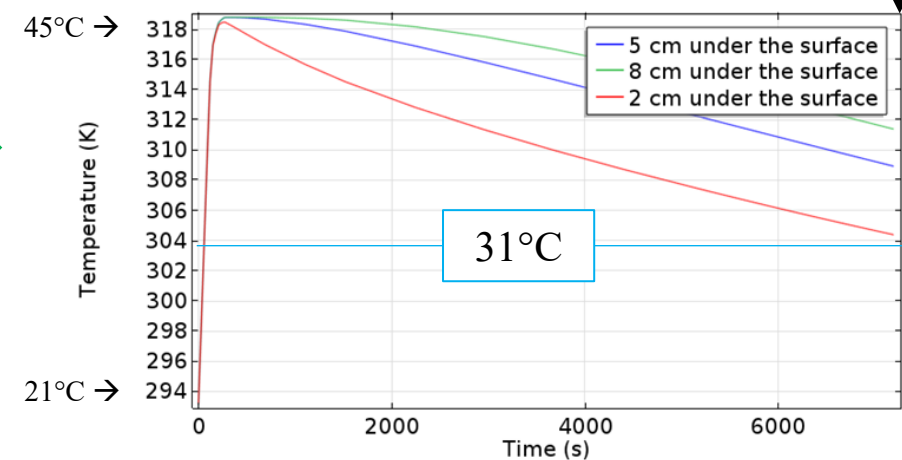
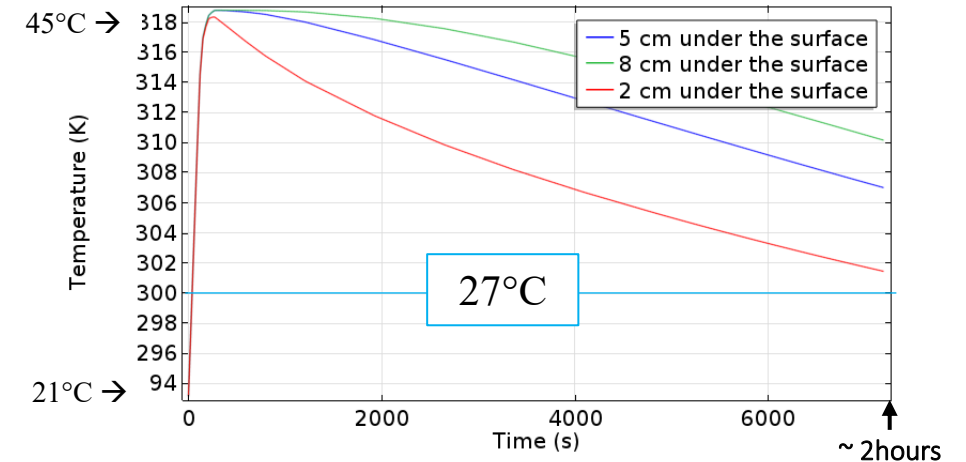
Heating

Emulsion + Quicklime Effects of temperature

FEM MODEL [tetrahedral elements]



Improving the insulation of the truck
→ + 4°C avg. temperature



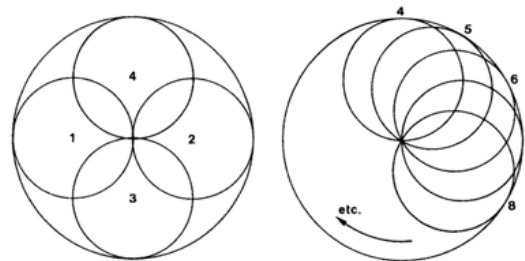
Compaction of BSM

Compaction Energy Estimation

Assumptions:

- A literature-based approximation was adopted to compare compaction methods on an equivalent energy basis
- RAP behaves as a granular material upon compaction

Modified Proctor test (ASTM D1557)

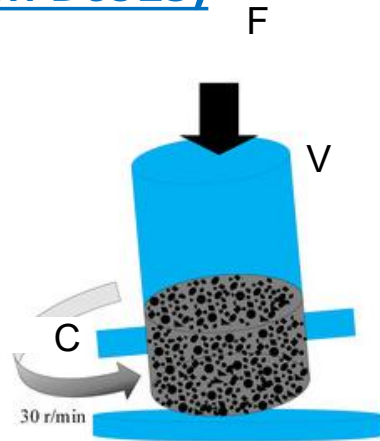
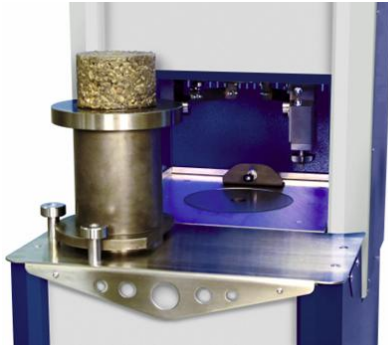


Compaction Method	Energy Application	Parameters Considered	Estimated Energy (kJ/m ³)
Modified Proctor	Impact Loading	Hammer weight (W_{mh}), number of blows (b), drop height (h), number of layers (l), volume of the mold (V)	$E_p = \frac{W_{mh} \cdot b \cdot h \cdot l}{1000 \cdot V}$

Compaction of BSM

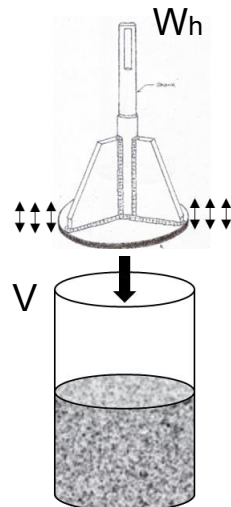
Compaction Energy Estimation

- Gyratory compactor (ASTM D6925)



Compaction Method	Energy Application	Parameters Considered	Estimated Energy (kJ/m³)
Gyratory compactor	Axial pressure + Rotational shearing	Pressure (F), height difference during one gyration (h), average torque (C), volume of the mold (V)	$E_g = \frac{F \cdot h + C \cdot (2 \cdot \pi)}{1000 \cdot V}$

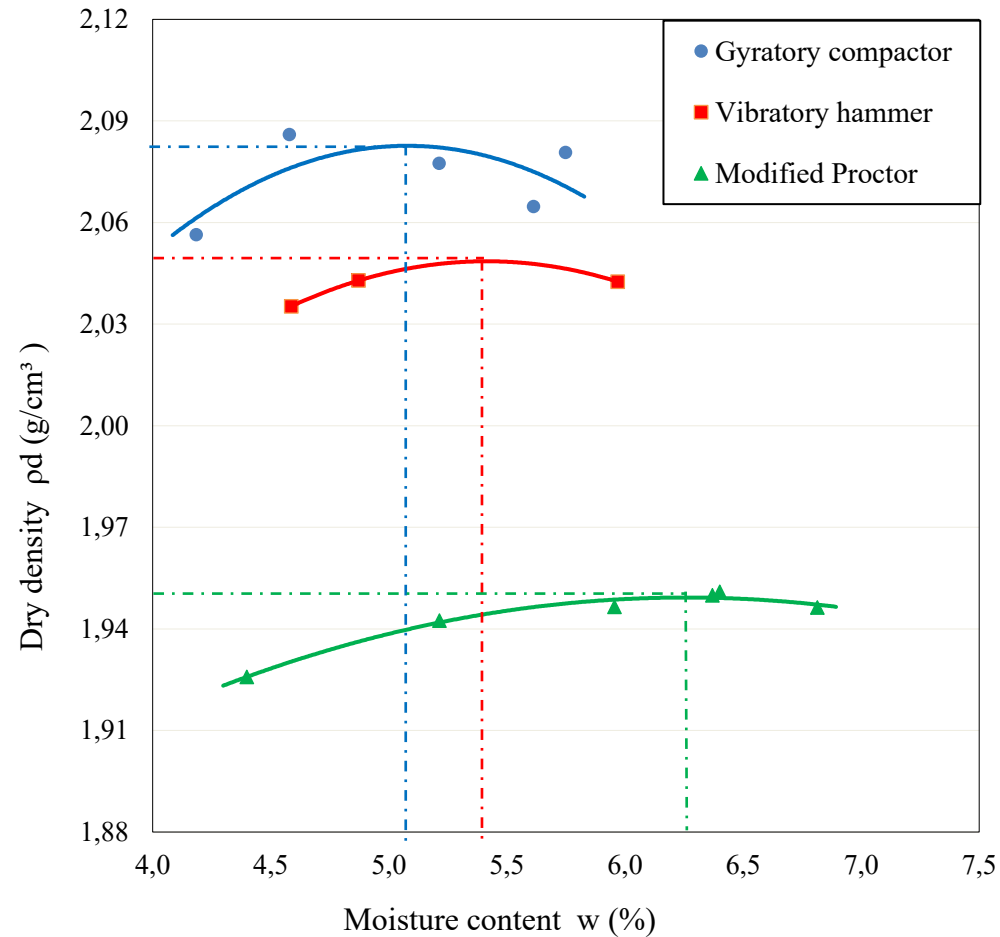
- Vibratory hammer



Compaction Method	Energy Application	Parameters Considered	Estimated Energy (kJ/m³)
Vibratory hammer	Axial pressure + Vibration	Hammer weight (W_h), frequency (f), amplitude (amp), compaction time (t), number of layers (l), volume of the mold (V)	$E_v = \frac{W_h \cdot f \cdot amp \cdot t \cdot l}{1000 \cdot V}$

Compaction of BSM

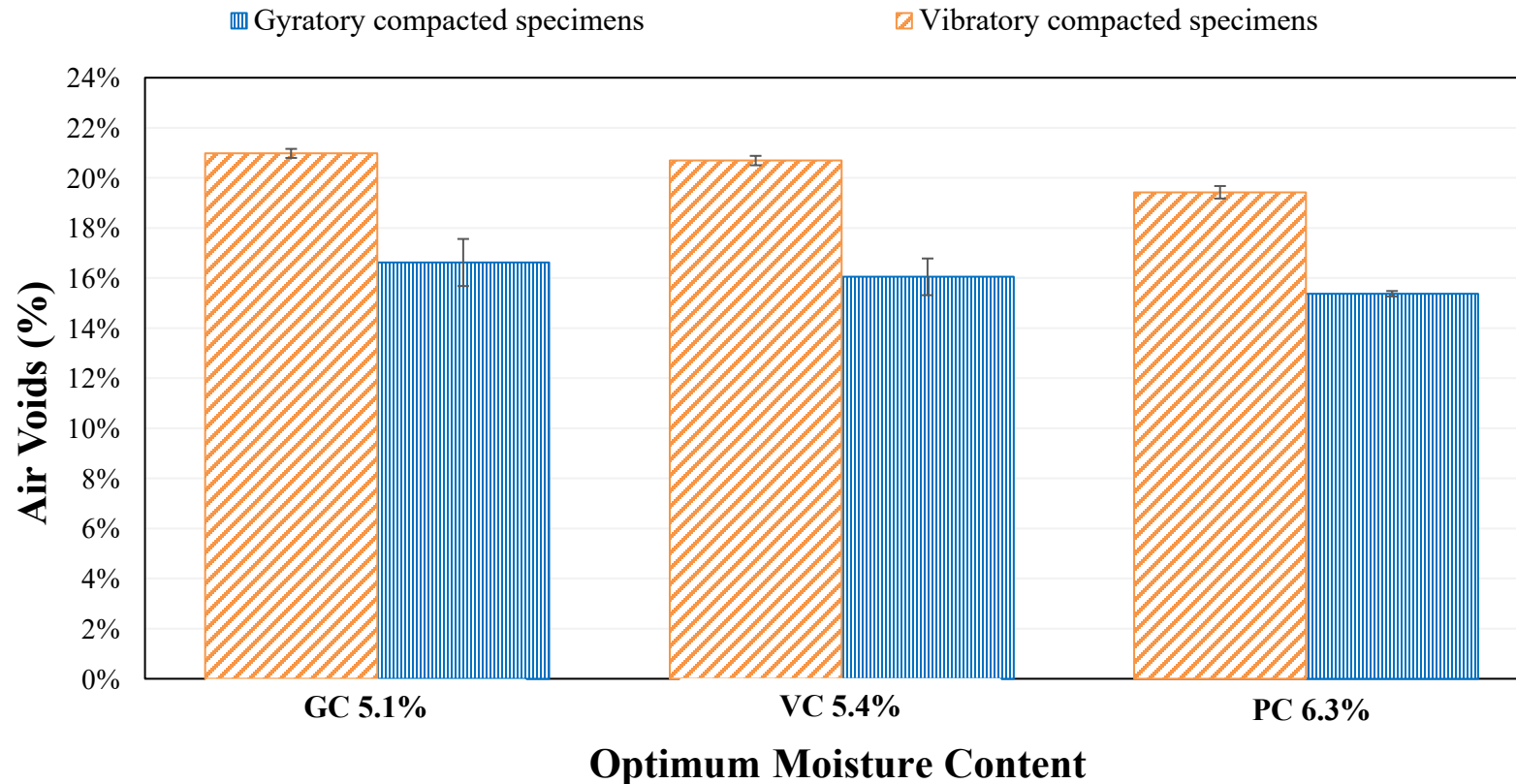
Different compaction energy application methods provide different density levels



	Compaction Method		
RAP Properties	Gyratory	Vibratory	Mod. Proctor
$\rho_{d,max}$ (g/cm³)	2.08	2.05	1.95
w_{opt} (%)	5.1	5.4	6.3
Estimated compaction energy (kJ/m³)	2289	2153	1957
Label	GC (5.1%)	VC (5.4%)	PC (6.3%)

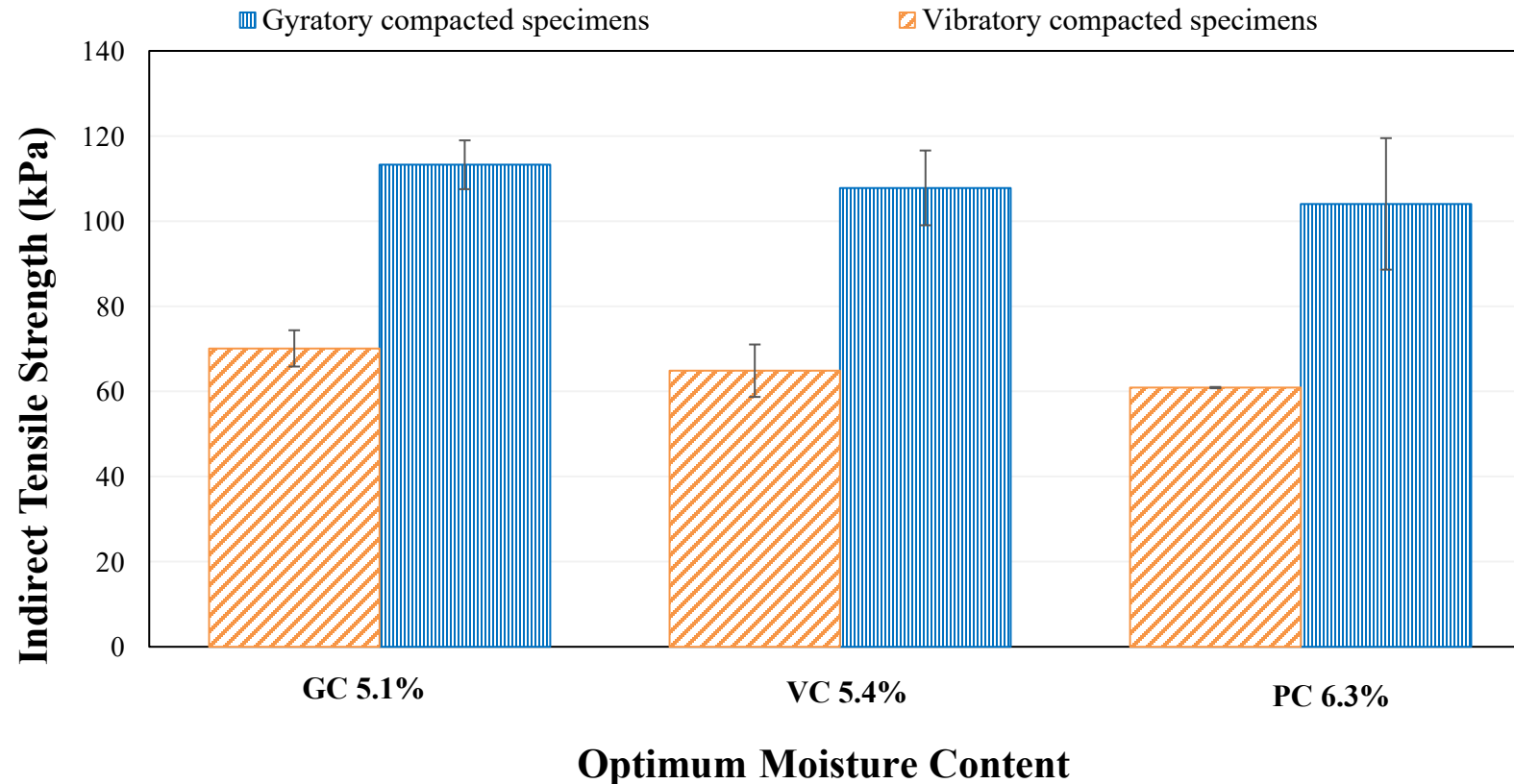
Compaction of BSM: Residual Air Void Content

- Air voids are always higher when vibratory hammer is used as compaction method
- % air voids decreases as the OMC increases, regardless of the compaction method



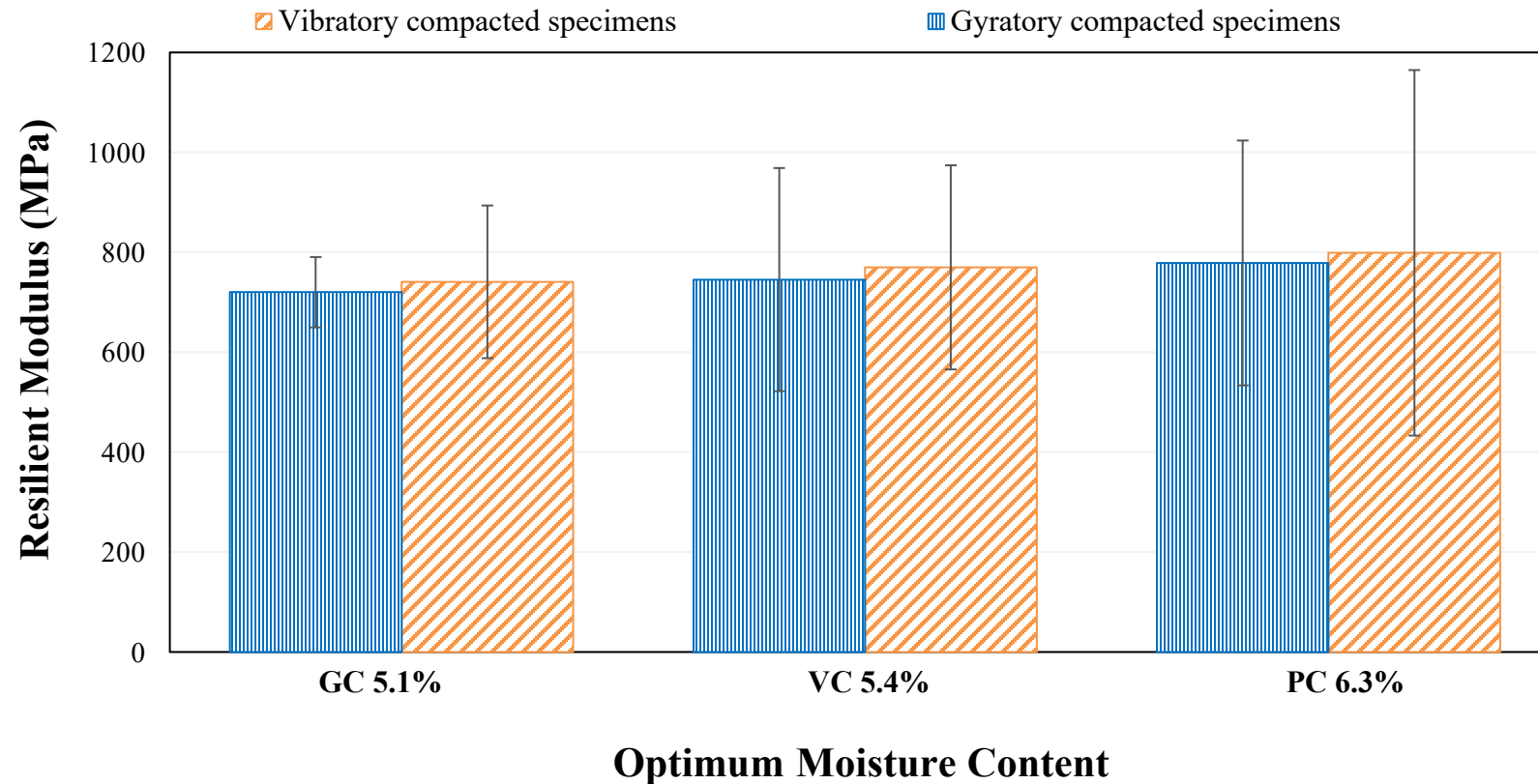
Compaction of BSM: Indirect Tensile Strength

- ITS is significantly higher when gyratory compactor is used as compaction method
- ITS slight decreases as the OMC increase, regardless of compaction method
- Highest ITS observed in the case of the lowest OMC (GC5.1% which has the highest air void content)



Resilient Modulus (MR)

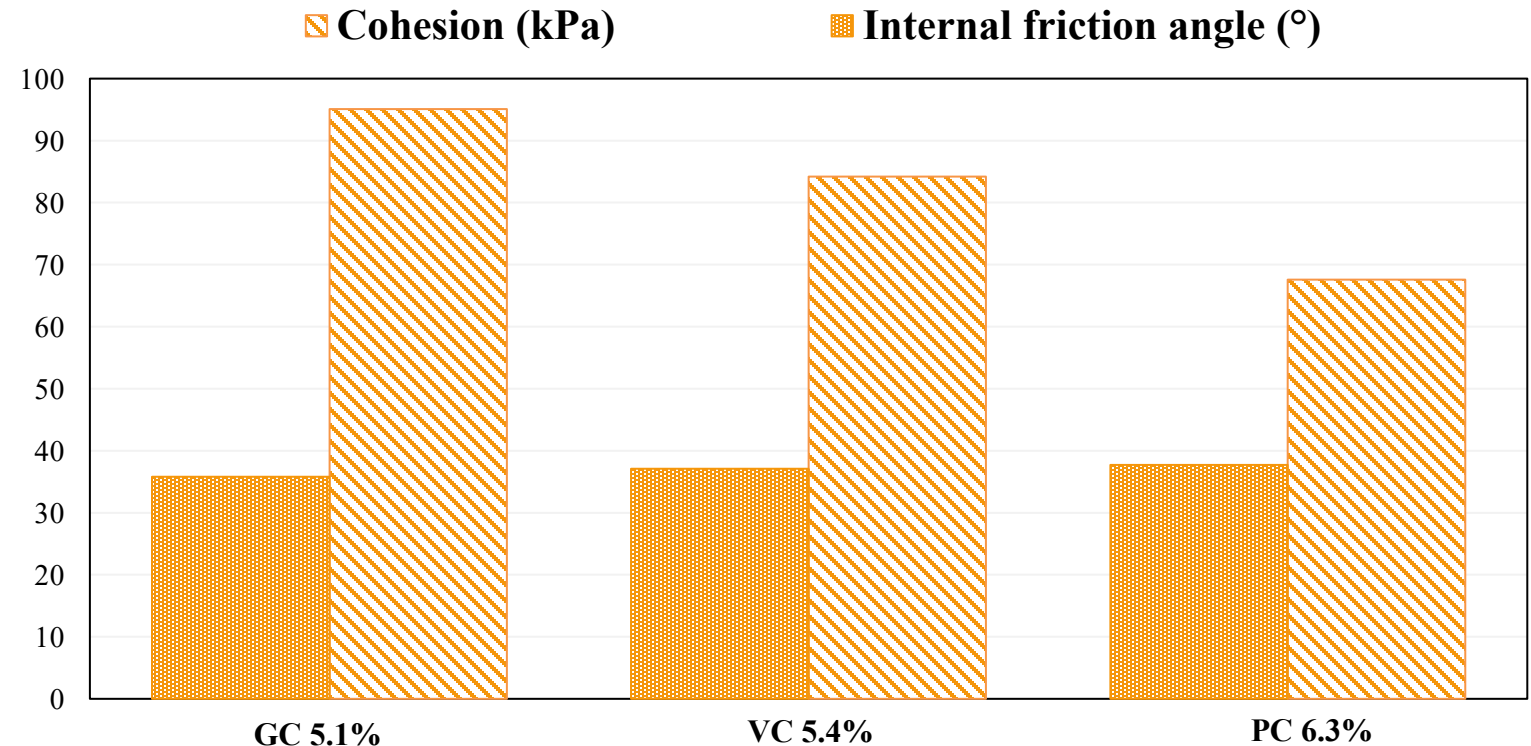
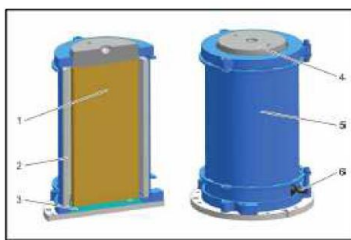
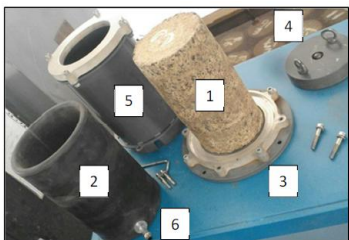
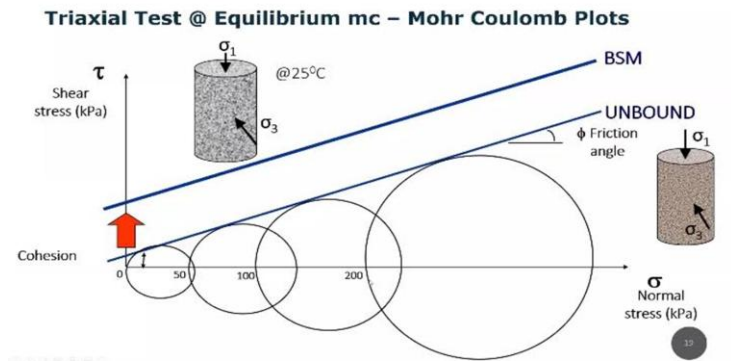
- Minimal variation as moisture content and compaction method vary
- Always higher when vibratory hammer is used as compaction method
- Increase as the OMC increases and air void content decreases



Compaction of BSM: Triaxial Shear Strength

- Cohesion decreases significantly as moisture content increases
- Internal friction angle varies in a narrow range
- GC 5.1% → highest cohesion and lowest internal friction angle

Specimens
compacted with
vibratory hammer



Optimum Moisture Content

Design of Pavements with BSM layer

Design with elasticity and plasticity

3D Elastic-Perfectly Plastic Finite Element Model

Elastic response:

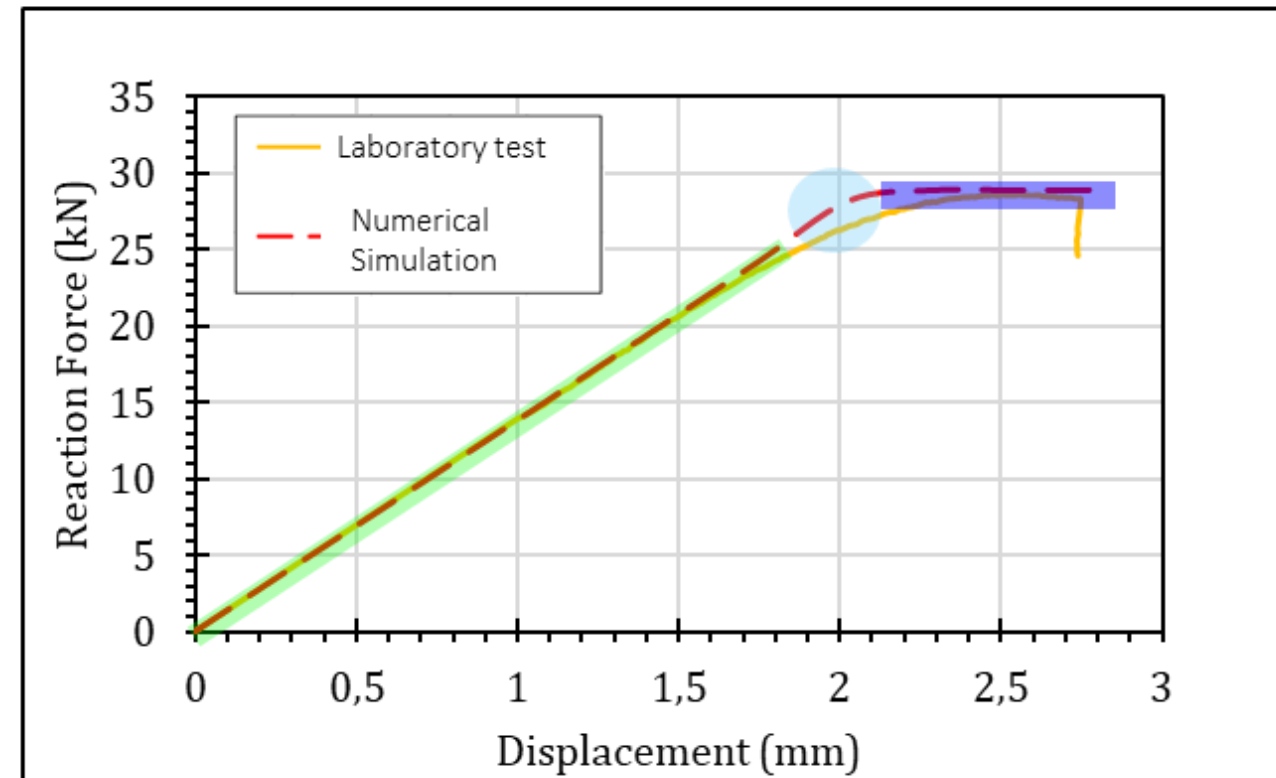
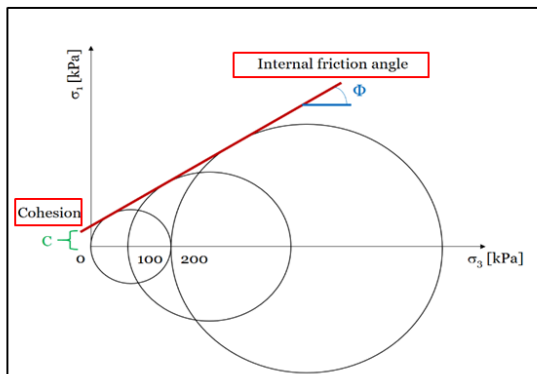
governed by Young's Modulus and Poisson's ratio.

Plastic response:

1. Criteria determining when plasticity gets initiated.
2. Flow rule dictating how much plasticity happens. (determines the extent of plastic strain for given stress state)

Selected criteria:

Mohr-Coulomb failure envelope



Design of Pavements with BSM layer

Design with elasticity and plasticity

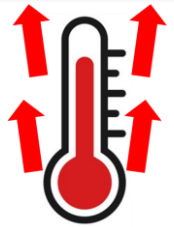
Layer	Thin pavement	Reference (TH 30)	Thick pavement
HMA	38 mm	75 mm	100 mm
BSM	50 mm	75 mm	100 mm
Granular Base	100 mm	250 mm	300 mm
Subgrade	Semi-infinite	Semi-infinite	Semi-infinite

Temperature: 10, 25 and 40 °C constant throughout HMA and BSM layers (temperature dependent materials)

Especially for thin pavement structures and high temperatures linear elastic analysis leads to overestimated pavement service life

Temperature [°C]	Structure	Model Type	Difference in fatigue life prediction (N_F) [%]	Difference in fatigue life prediction (N_R) [%]
10	Thin	Linear elastic	-64.56	-47.62
		Plasticity-based		
25	Thin	Linear elastic	-75.75	-54.14
		Plasticity-based		

Hot Asphalt Recycling



Hot recycling



Aggregates



+

&



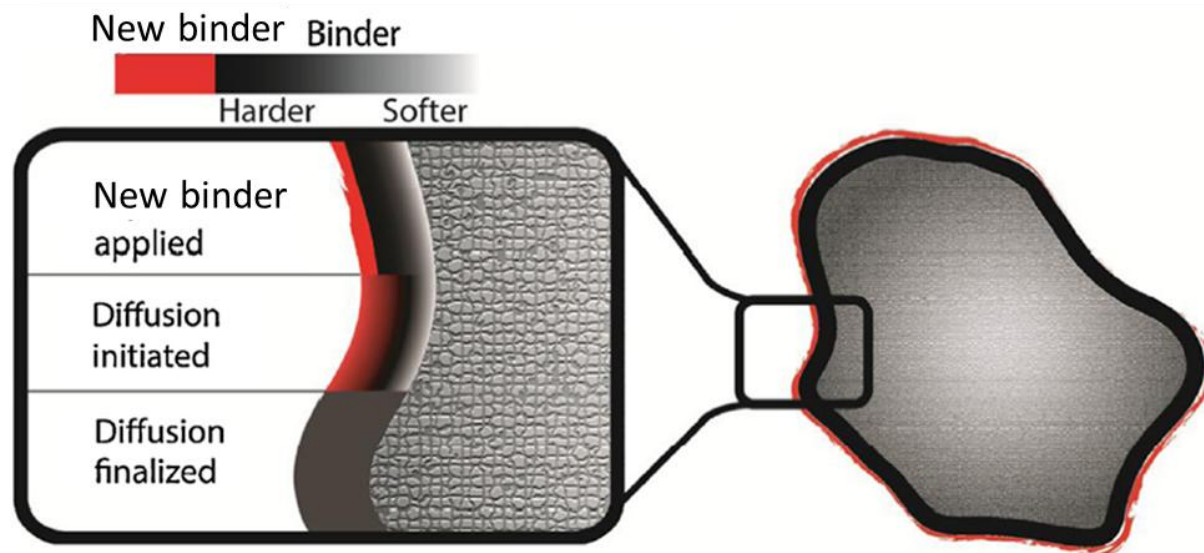
RA's bitumen



fresh bitumen

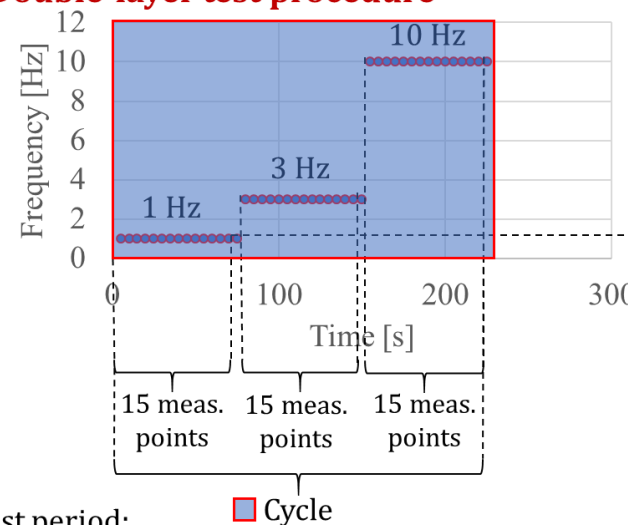


Hot Asphalt Recycling: Diffusion

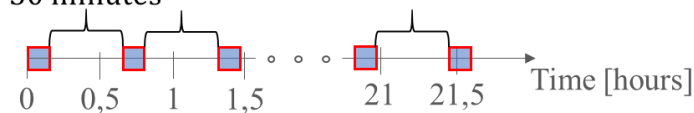


Courtesy of Jan Krol

Double-layer test procedure



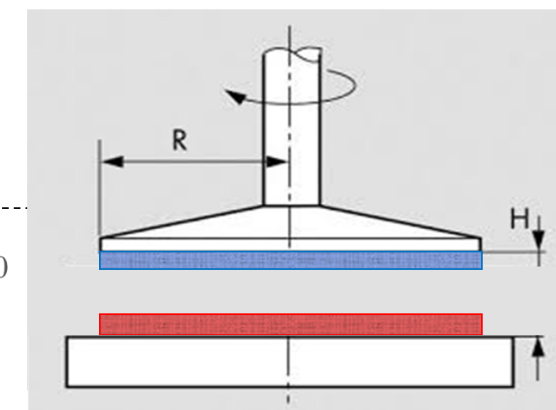
Rest period:
30 minutes



38 cycles

→

Test duration ≈ between 21 to 22 hours



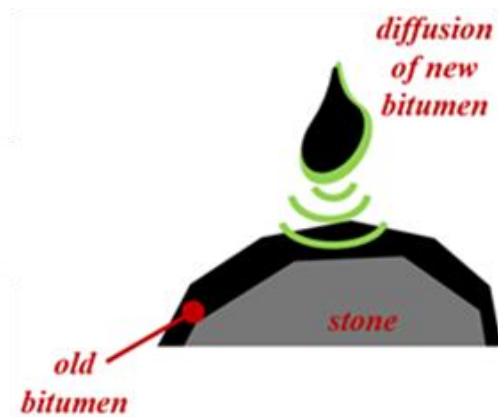
$T = 50^{\circ}C$ $H = 0.5mm$ $\gamma = 0.5\%$



Trimming of the two samples,
after the conditioning at $20^{\circ}C$.
Sample thickness: 0.5mm

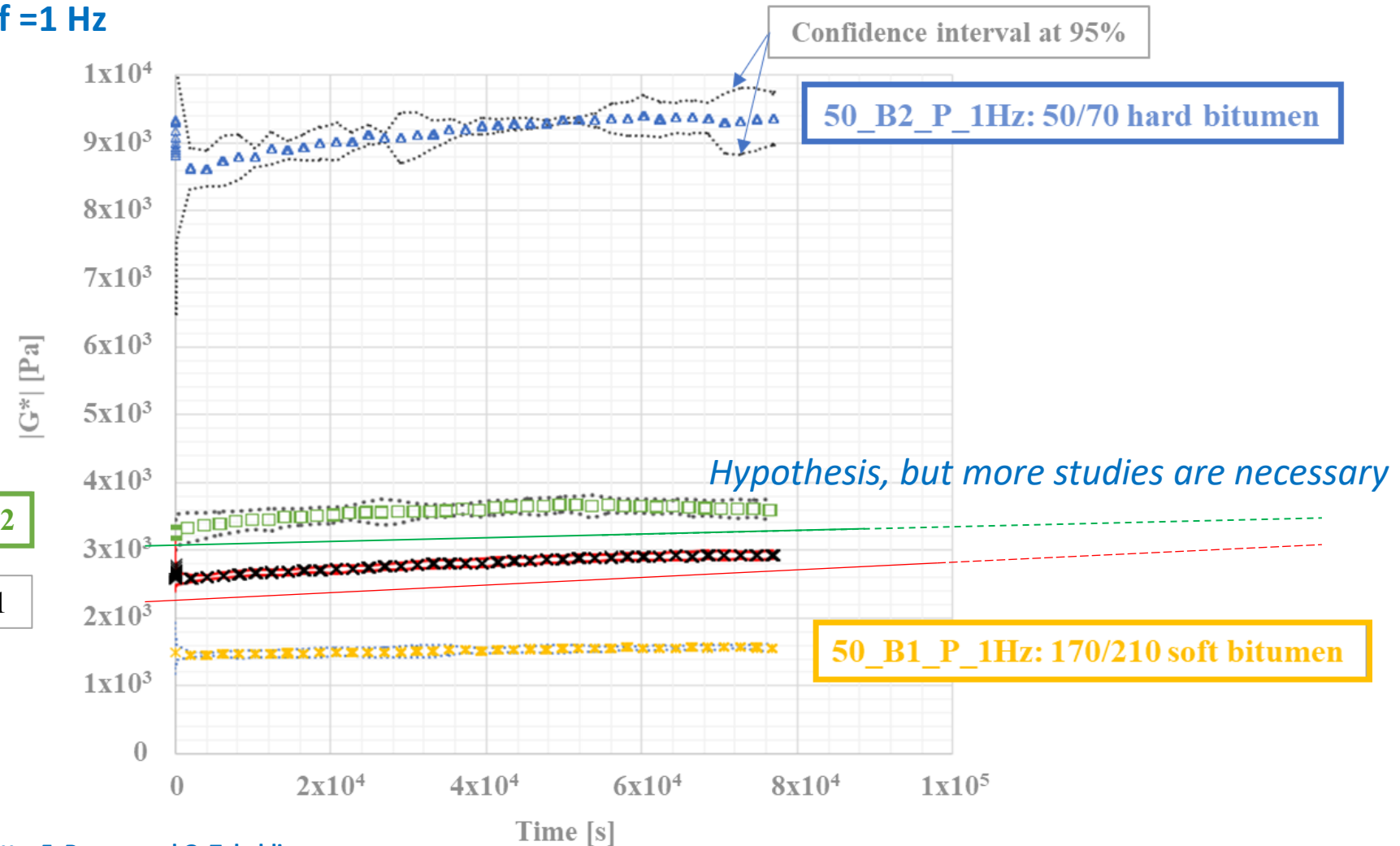
Hot Asphalt Recycling: Diffusion

Diffusion tests B1/B2 at 50°C – $\gamma = 0,5$; $f = 1$ Hz



50_B3_P_1Hz: full blend 50%B1+50%B2

50_B2_B1_P_1Hz: double layer B2+B1



Hot Asphalt Recycling: Diffusion

- $G^* = |G^*|e^{i\varphi}$
- $x(t)$ complex number

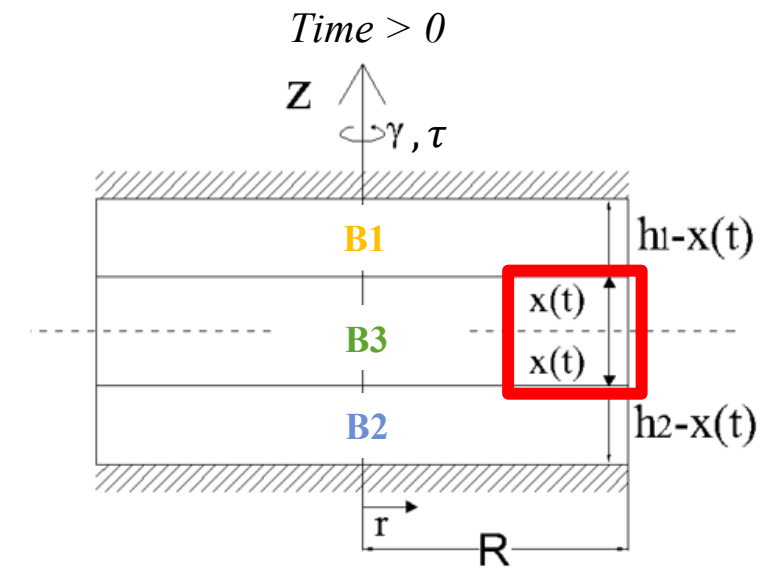
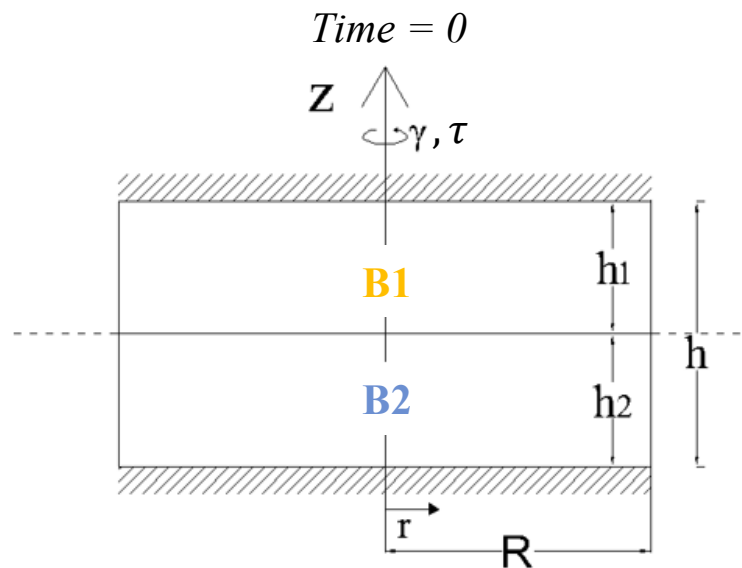
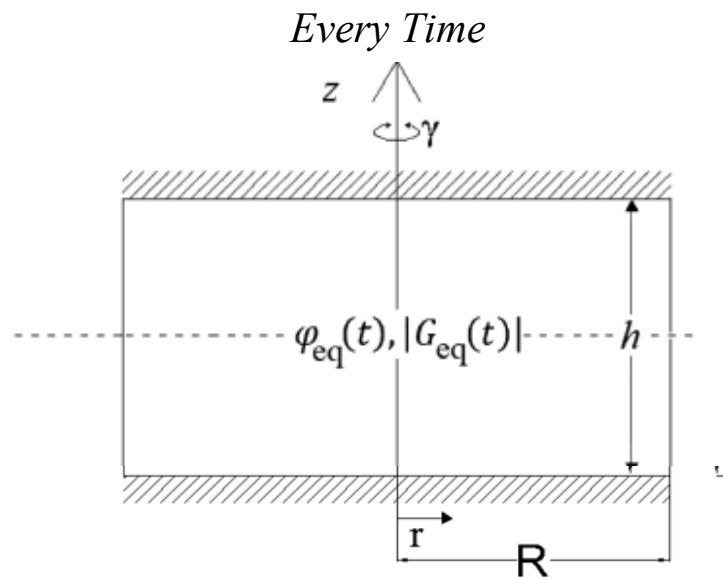
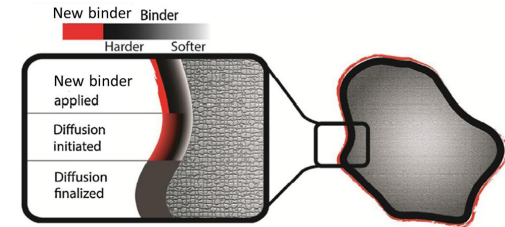
Bitumen B1

Bitumen B2

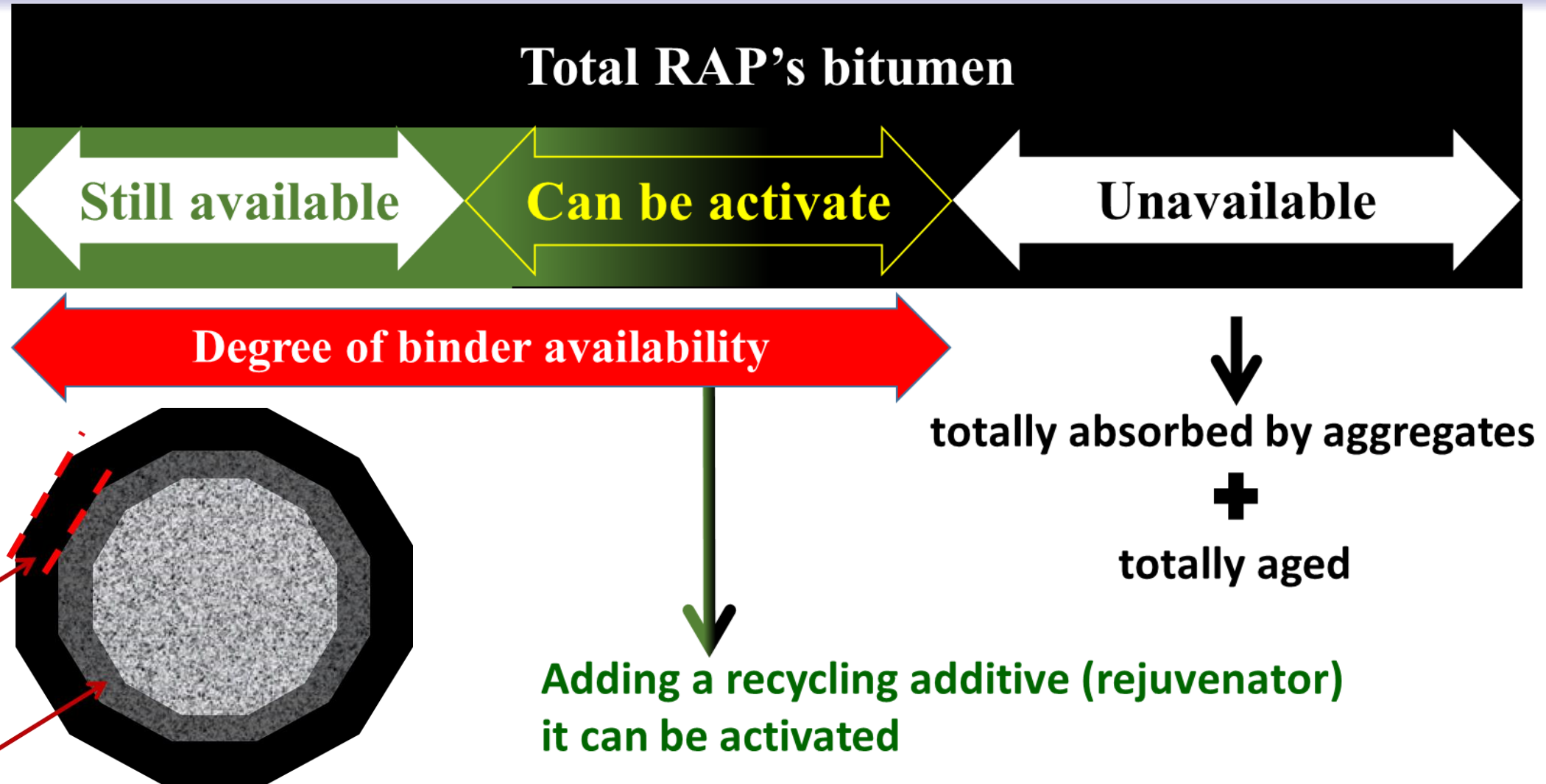
Bitumen B3 full-blend B1+B2

Estimate of diffusion thickness

$$\frac{h}{G_{eq}^*(t)} = \frac{h_1 - x(t)}{G_1^*(t)} + \frac{h_2 - x(t)}{G_2^*(t)} + \frac{2x(t)}{G_3^*(t)}$$

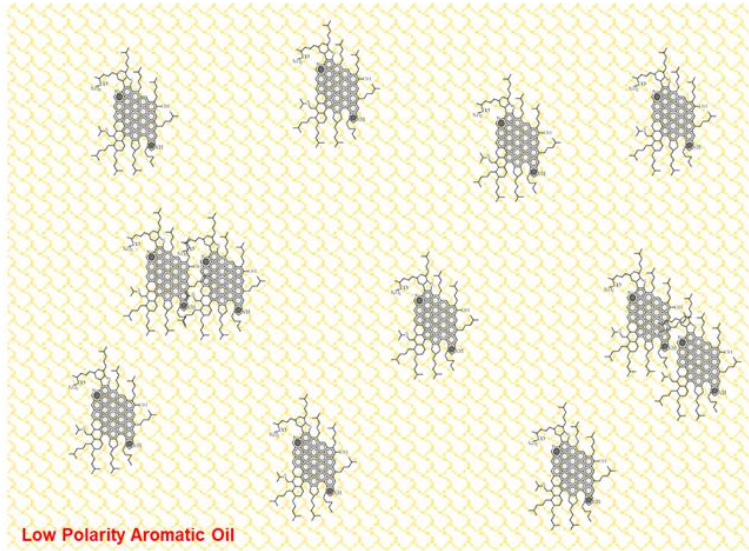


Hot Asphalt Recycling: Binder Availability

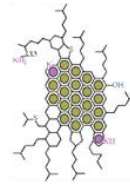


Hot Asphalt Recycling: R.A. action

Unaged Bitumen



Asphaltene



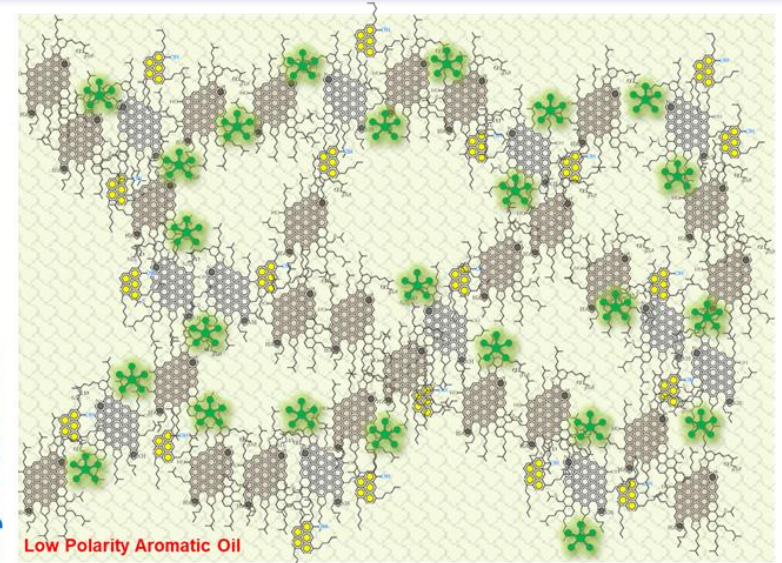
Resins



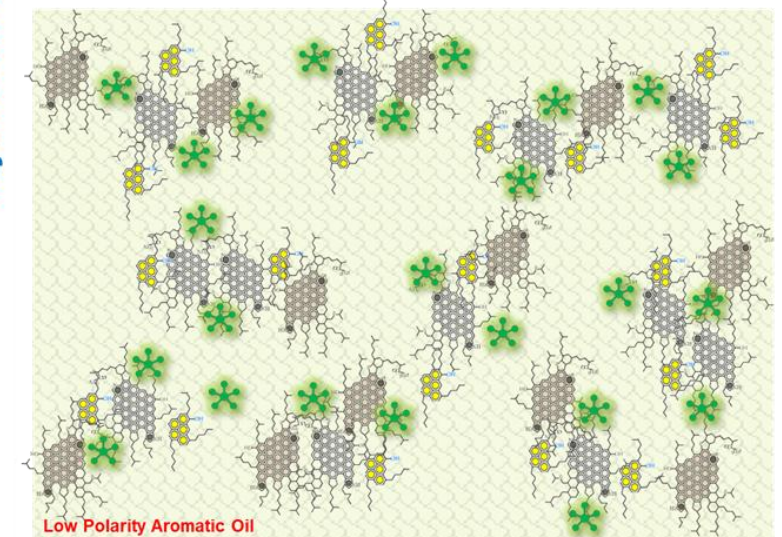
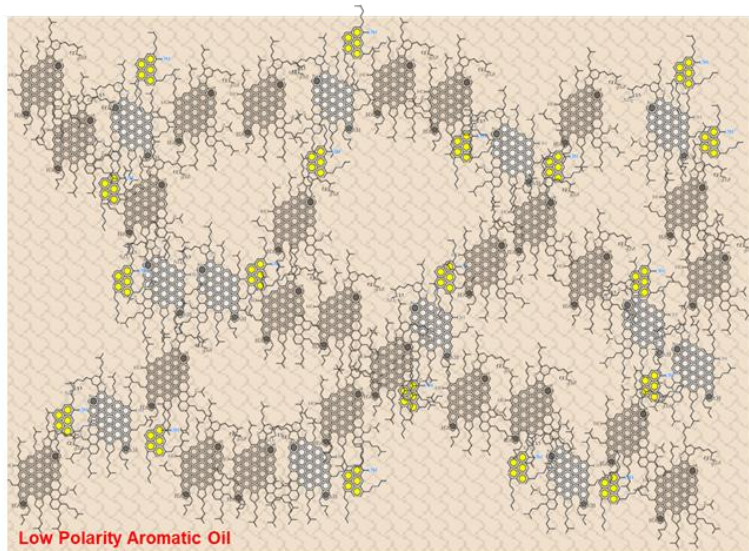
Rejuvenator



Rejuvenation of bitumen

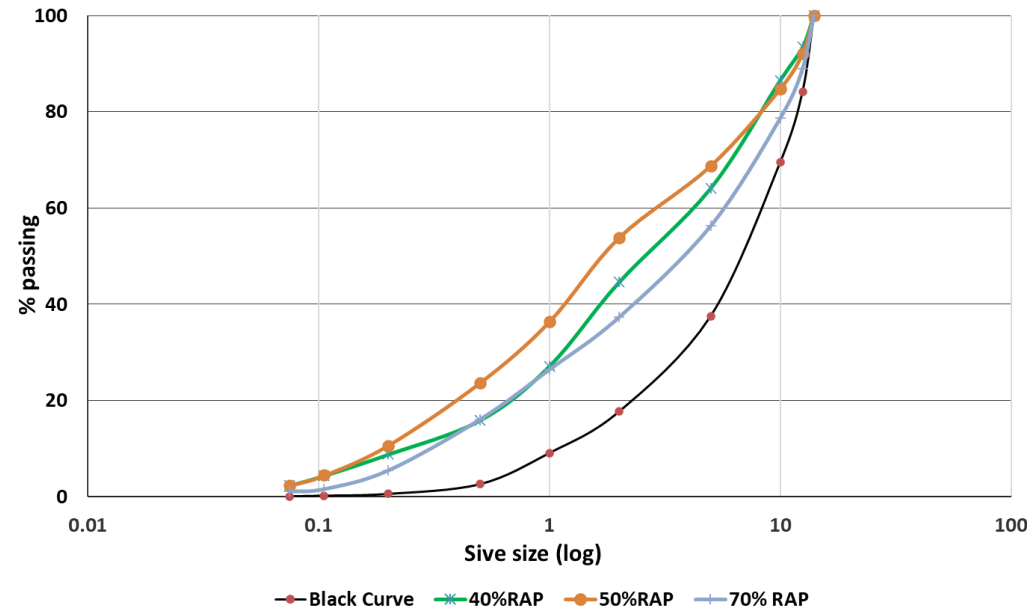


Aged Bitumen



Courtesy of Dr. Hans Moolenaar

Hot Asphalt Recycling: High amount of RAP

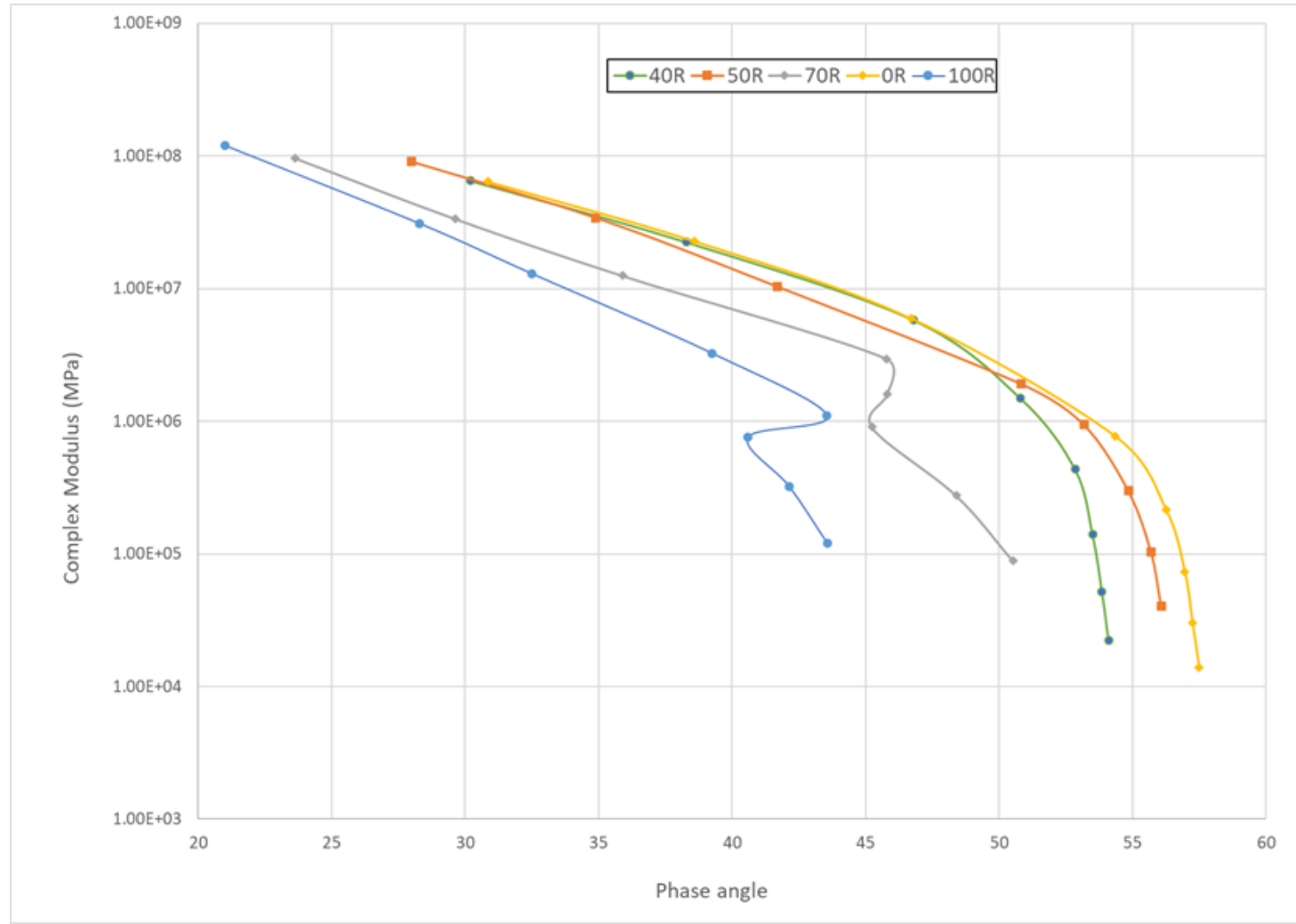


Label	Virgin aggregates (%)	RAP (%)	Asphalt (%)	Bio-oil (%)
0R	100	-	5.2	-
20R	80	20	5.2	-
40R	60	40	3.3	0.11
70R	30	70	1.8	0.22

Dosage of R.A. based on viscosity

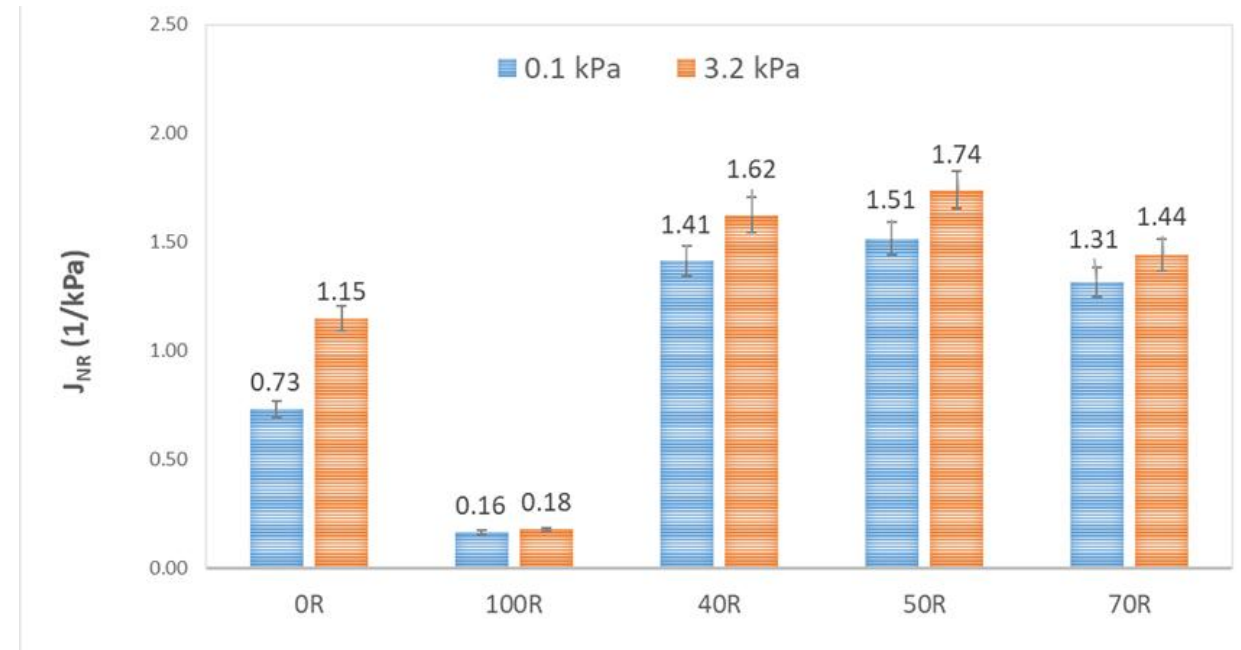
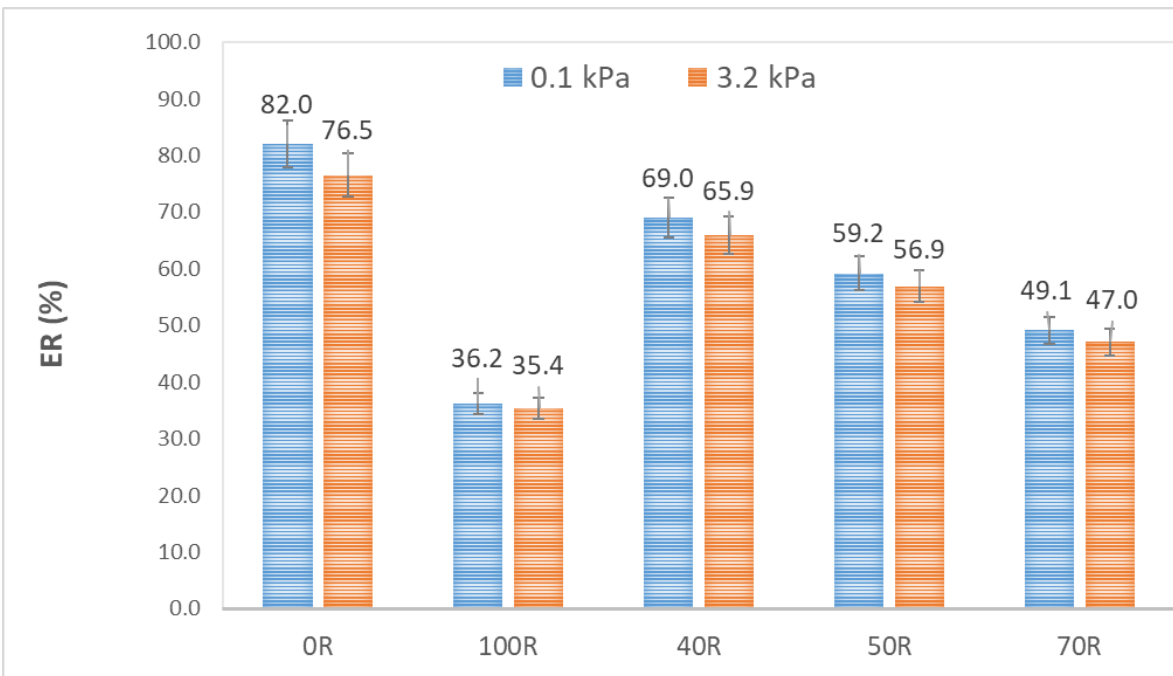
Hot Asphalt Recycling: High amount of RAP

Full Blending: Black Space @10rad/s



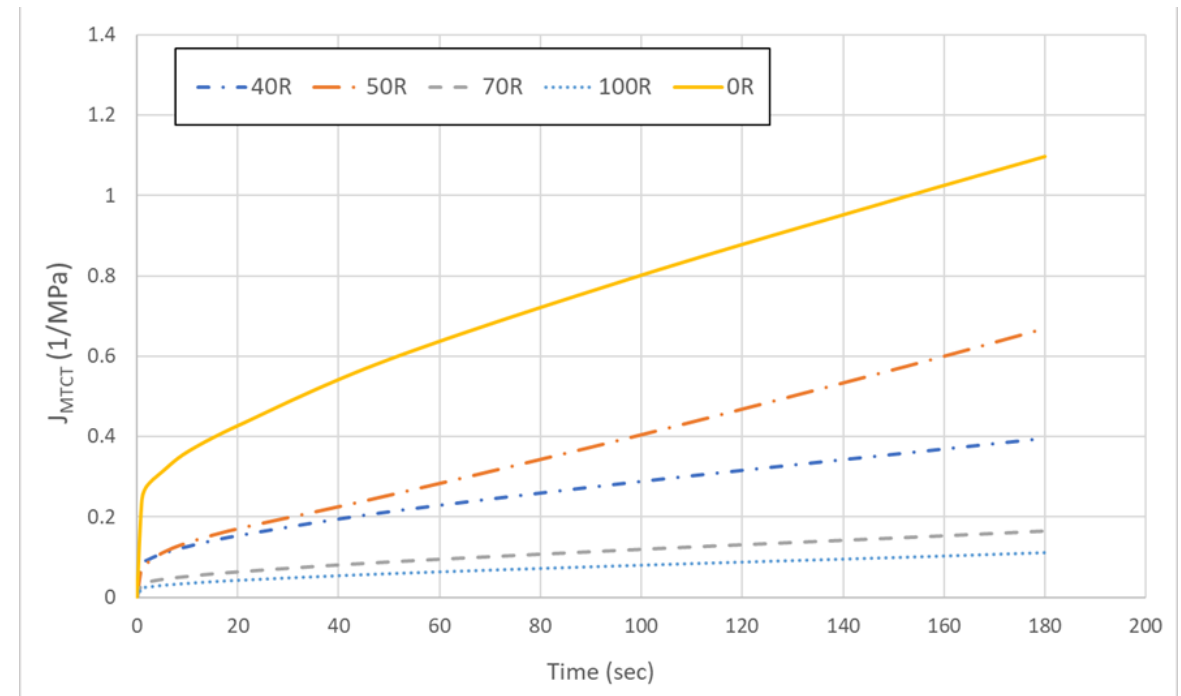
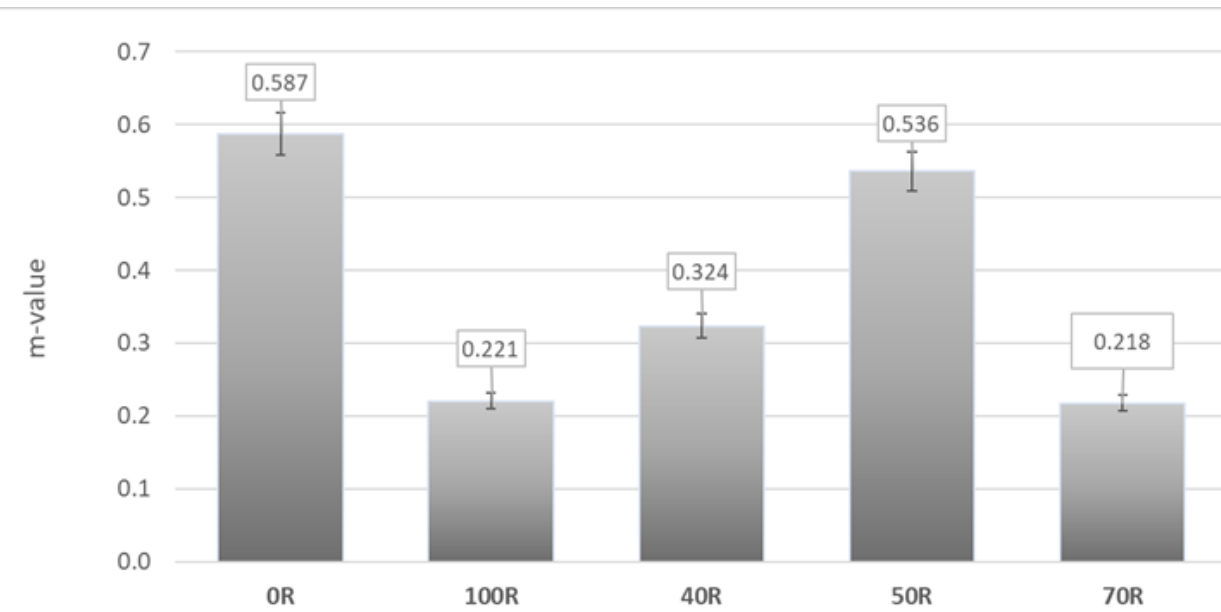
Hot Asphalt Recycling: High amount of RAP

Full Blending: Elastic Recovery & Non-Recoverable Creep Compliance



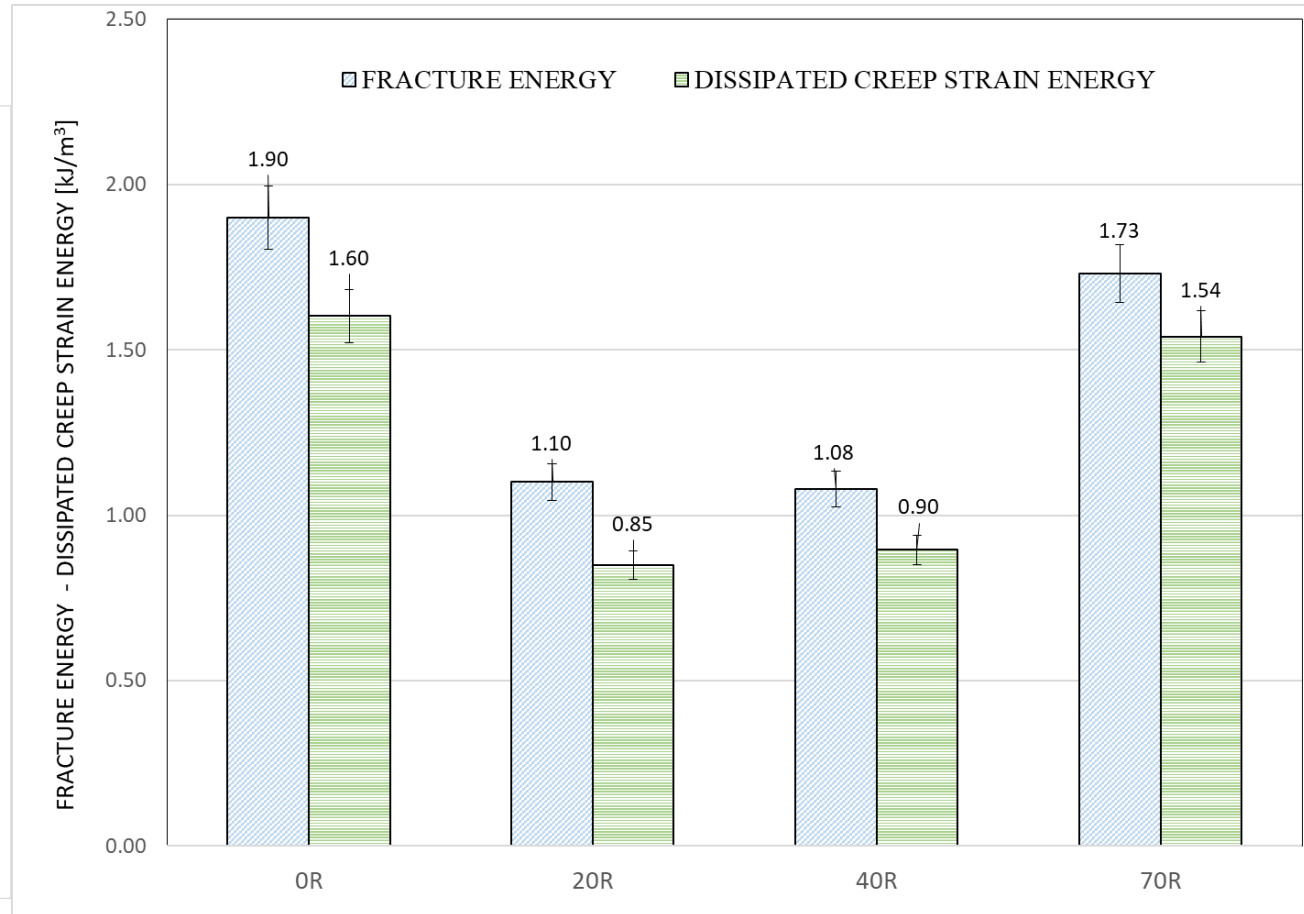
Hot Asphalt Recycling: High amount of RAP

Mastics' Creep Compliance @10°C



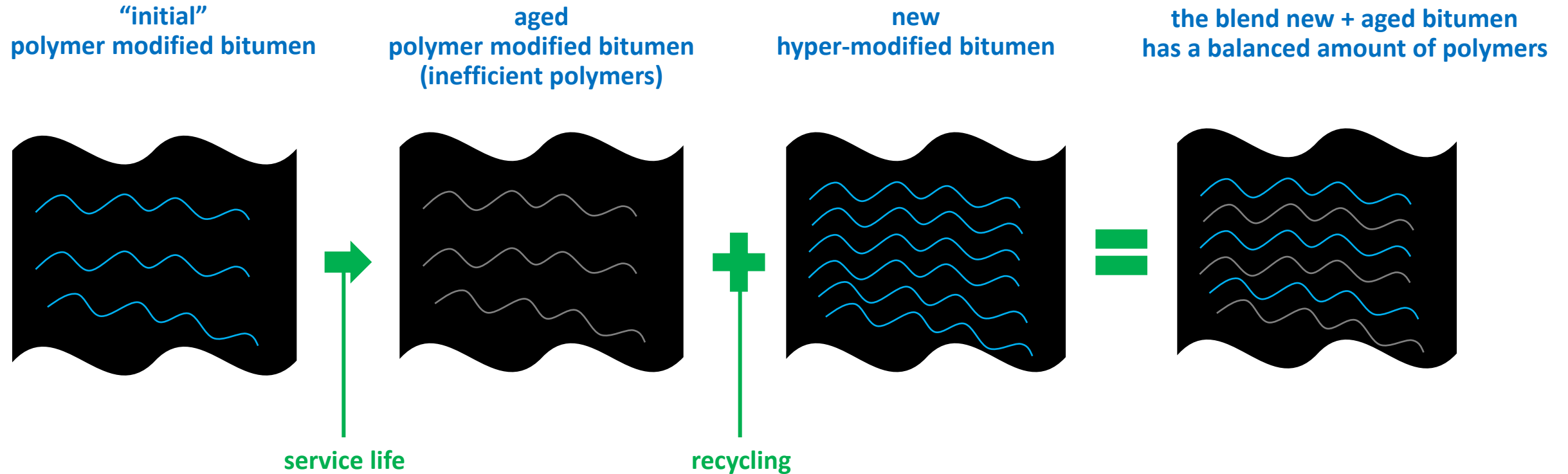
Hot Asphalt Recycling: High amount of RAP

SUPERPAVE IDT & HMA Fracture Mechanics analysis



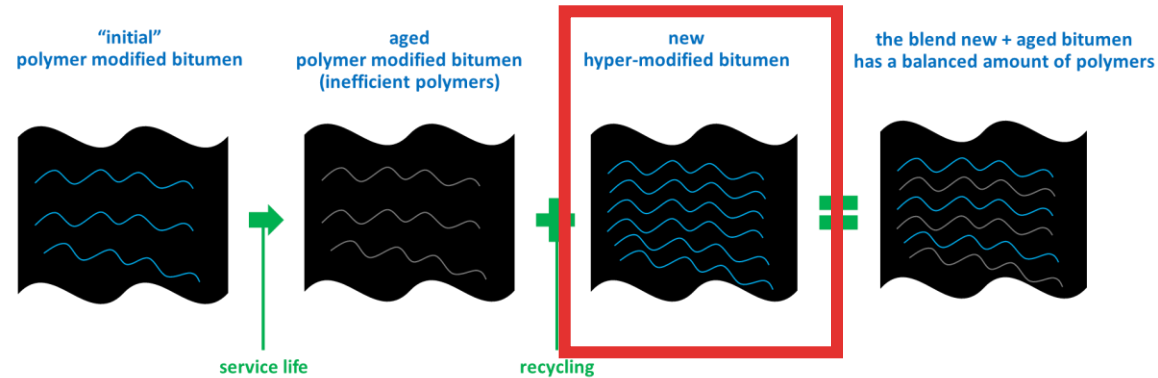
Hot Asphalt Recycling & Polymer level

Diffusion & blending can help to keep the appropriate level of polymers



Hot Asphalt Recycling & Polymer level

Hyper-modified bitumen

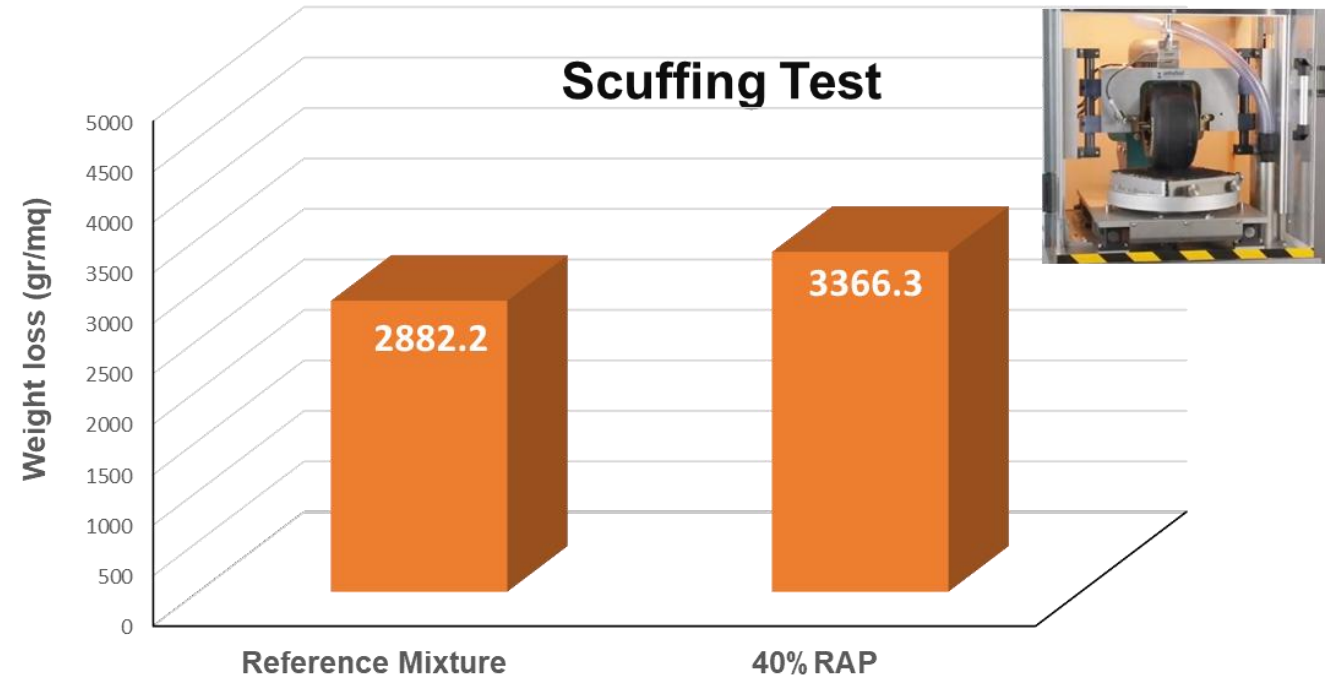
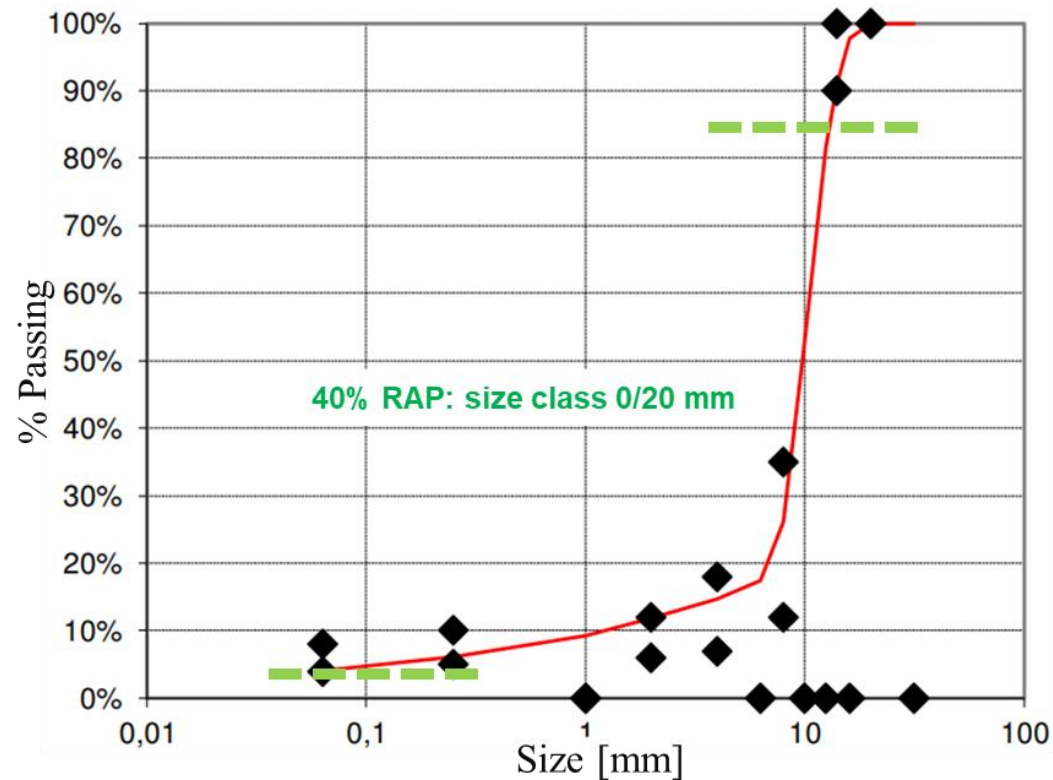


Pen@25°C [UNI EN 1426]	25÷55 dmm
R&B [UNI EN 1427]	>85°C
Elastic recovery @25°C [UNI EN 13398]	≥ 95%
Dynamic Viscosity @100°C [UNI EN 13302]	> 80Pa·s
Dynamic Viscosity @160°C [UNI EN13302]	> 0,8Pa·s
Polymer content (on the bitumen weight)	>7%
Evaluation of Storage Stability (tuben test) [UNI EN 13399]	Δ Pen@25°C < 9.0dmm
	Δ R&B < 5.0°C
Δ Pen@25°C after RTFOT	< 40%
Δ R&B after RTFOT	< 5°C

Hot Asphalt Recycling: R.A. & HyMB

OGFC: 40% RAP - 60% natural aggregates

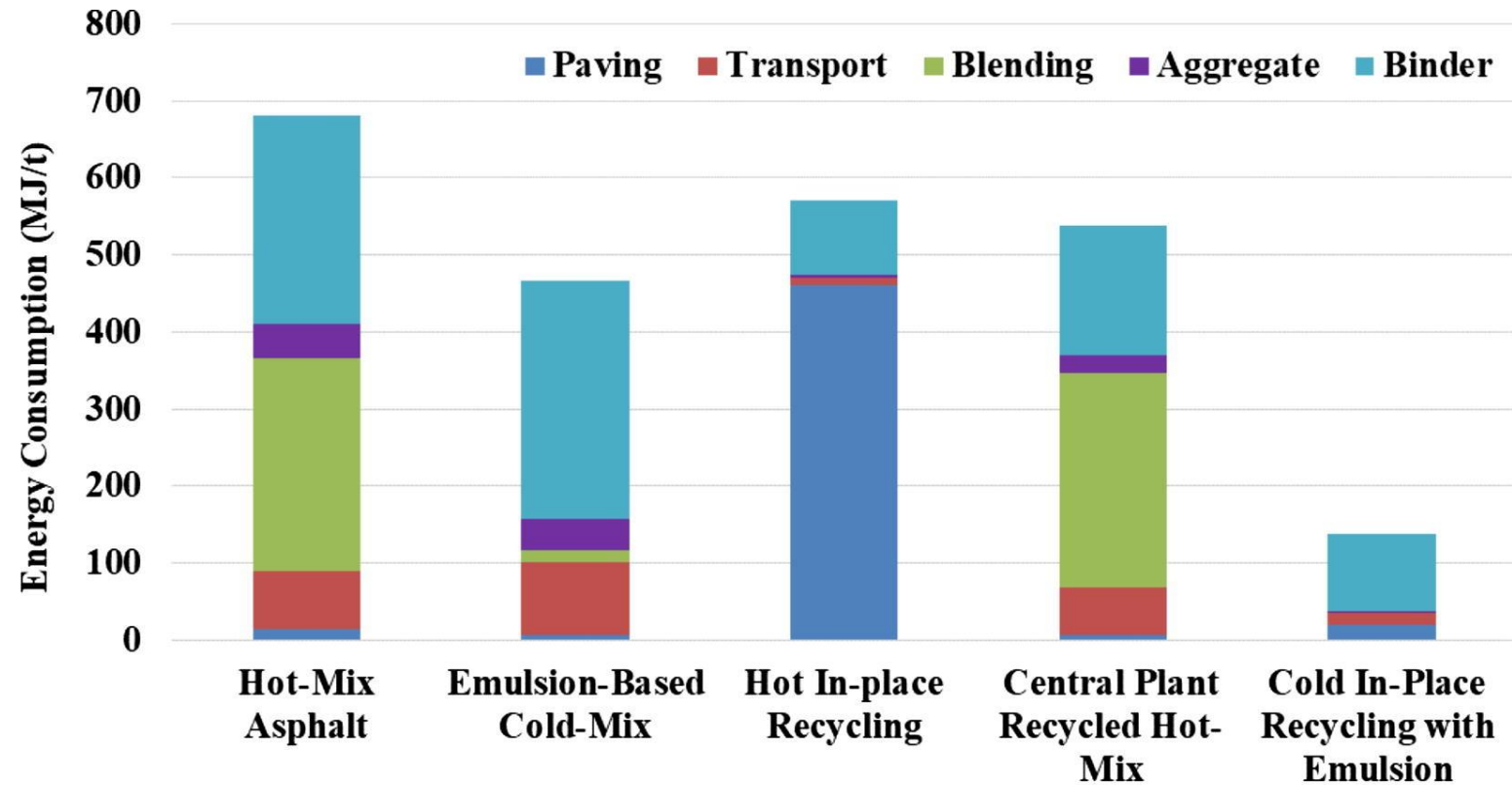
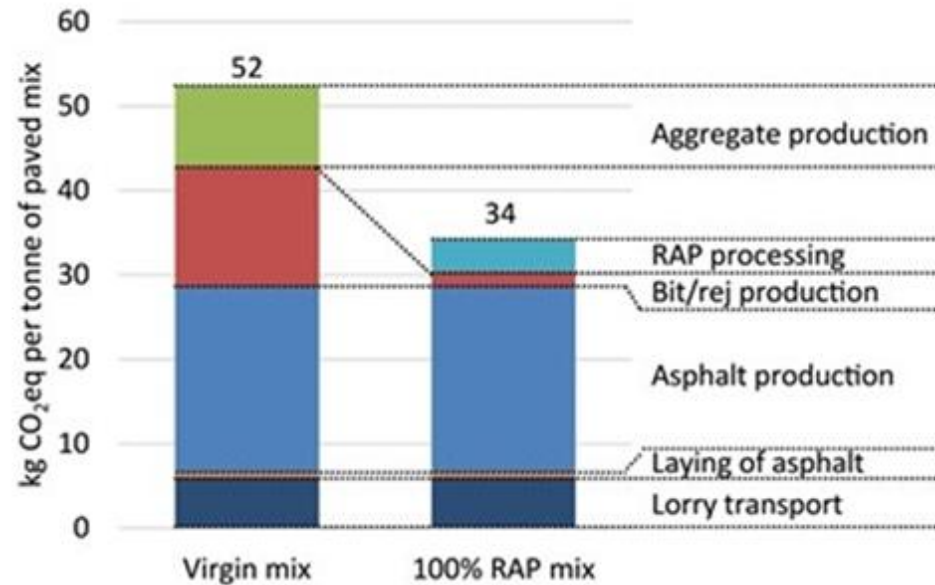
The total amount of RAP is due to: asphalt plant characteristics (single drum), agency requirements, Size distribution and aggregates' maximum size of mixtures that generated RAP, "toughness" of RAP particles



Dosage of Recycling Agent: 0.13% w/w mixture

Total amount of bitumen: 5% (additional bitumen: hyper modified)

Recycling & Sustainability



Conclusions & Remarks

COLD RECYCLCLING

- Optimization of specimen preparation techniques (compaction, curing,..)
- “Smart” use of active fillers
- Proper pavement design approach

HOT RECYCLCLING

- Maximization of the amount of RAP
- Full understand of blending and diffusion mechanism
- Proper approach for Recycling Agents dosage → Rheological approach

2nd RILEM International Symposium on Bituminous Materials

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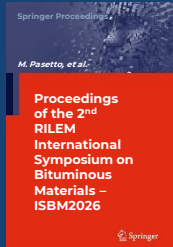


To be finalized



2nd RILEM International Symposium on Bituminous Materials

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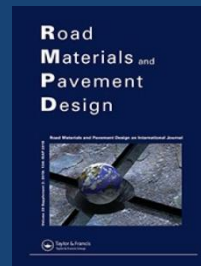
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